

**VILLAGE OF DOWNERS GROVE
PLAN COMMISSION**

VILLAGE HALL COUNCIL CHAMBERS
801 BURLINGTON AVENUE

February 5, 2018
7:00 p.m.

AGENDA

1. Call to Order

a. Pledge of Allegiance

2. Roll Call

3. Approval of Minutes – January 8, 2018

4. Public Hearings

- a. **17-PLC-0014:** A petition seeking approval of a Planned Unit Development, Special Use for a fueling station and a drive-through for a restaurant, and an alley vacation. The property is zoned B-3, General Services and Highway Business. The property is located at the southeast corner at the intersection of Ogden Avenue and Belmont Road, commonly known as 2125 Ogden Avenue, Downers Grove, IL (PIN 08-01-405-042). C.M. Lavoie & Associates, Petitioner and Powermart Real Estate Downers Grove #3, LLC, Owner.
- b. **17-PLC-0041:** A petition seeking approval of a Planned Unit Development Amendment to construct a new convenience goods store, a Special Use for a drive-through facility, and a Plat of Subdivision. The property is zoned B-2/PUD, General Retail Business/Planned Unit Development. The property is located at the southwest corner of 63rd Street & Woodward Avenue, commonly known as 2001 63rd Street, Downers Grove, IL (PINs 08-24-202-008, -009). FL Cedar, LLC, Petitioner and Owner.

5. Adjournment

THIS TENTATIVE REGULAR AGENDA MAY BE SUBJECT TO CHANGE

**VILLAGE OF DOWNERS GROVE
PLAN COMMISSION MEETING**

MINUTES FOR JANUARY 8, 2018

In the absence of Chairman Rickard, Ms. Gassen served as Chairperson ProTem (hereinafter Ch.) and called the January 8, 2018 meeting of the Plan Commission to order at 7:00 p.m.

ROLL CALL:

PRESENT: Mr. Boyle, Ms. Gassen, Ms. Johnson, Mr. Kulovany, Mr. Maurer, Ms. Rollins

ABSENT: Ch. Rickard, Mr. Quirk, Ex. Officio members Miller, Livorsi & Menninga

STAFF: Sr. Village Planner Rebecca Leitschuh
Village Planner Scott Williams

VISITORS: Amy Fuller, Wight & Company
Byron Wynn, Wight & Company Land Development Group
Jo Potts, Lester Neighbor, 216 Lincoln
Cindy Gilbert, 240 Lincoln
The Hartnetts, Lester Neighbor, 300 Lincoln
Carolyn Quinn, Lester Neighbor, 4615 Fairview Ave.
Anas Alkhatib, Damas Consulting Group, 5625 Middaugh
Paul Chabez, Jr., Phorma Designs Inc. 2092 Gardner Cir. E., Aur

Ch. Gassen requested that all cell phones be silenced during the meeting, and informed the public that copies of the Agenda were available at either side of the Council Chambers.

APPROVAL OF MINUTES: December 4, 2017 meeting

Mr. Kulovany moved, seconded by Mr. Maurer to approve the minutes for the December 4, 2017 meeting.

The Motion to approve the minutes as presented passed by voice vote with Mr. Boyle abstaining.

Ch. Gassen reviewed the procedures to be followed for the meeting, explaining that the Plan Commission is a recommending body for the petitions on the Agenda. Once the Public Hearing portion of the meeting is closed, the Plan Commission members will deliberate to recommend approval, recommend approval with conditions, or recommend denial of the individual petition. That recommendation will be forwarded to the Village Council with the minutes, exhibits and all documentation from the public hearing on the specific petition. The Village Council will make final decisions at a future date. She

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reviewed the subject matter of the two petitions on the Agenda, and then asked all individuals intending to speak during either of the public hearings to rise and be sworn in.

17-PLC-0036: A petition seeking approval of a Zoning Map Amendment and a Special Use to allow a school addition. The property is currently zoned R-4, Residential Detached House 4, proposed to rezone to INP-2, Campus-Scale Institutional and Public. The property is located west of Cumnor Road, between Lincoln Avenue and Indianapolis Avenue, commonly known as 236 Indianapolis Avenue, Downers Grove, IL (PINs 09-04-303-011, 09-04-303-012, 09-04-308-001). Wight & Company, Petitioner; School District No. 58 Owner.

Mr. Scott Williams, Planner for the Village of Downers Grove, stated that the subject property borders Lincoln Avenue, Cumnor Road and Indianapolis Avenue and is currently zoned R-4, Residential Detached House 4. The petitioner is applying for the INP-2 zoning designation. He displayed site plans of the property, depicting a proposed 3400 square foot addition to be located at the playground area to the east of the site. The proposal will meet all bulk requirements. One curb cut will be removed and replaced with a walkway and landscaping. The petitioner has submitted a landscaping plan for the site, and there is a reduction of 1071 square feet of impervious area. The addition will connect into existing utility systems. This addition will allow for the return of the school's full-day kindergarten program. Elevation drawings show that the addition will meet height requirements and all mechanicals will be fully screened.

Mr. Williams explained that INP-2 zoning is intended to accommodate development and expansion of large public, civic and institutional uses while minimizing the potential for adverse impact on surrounding areas. He noted that the total elementary school property is approximately 6.7 acres in size and bordered by single-family residences on all sides. He noted that according to the Comprehensive Plan's Future Land Use Map, the footprint of the school is not being expanded beyond the initial perimeter of the property. The current land use will be consistent with the proposed zoning change.

Mr. Williams said that Staff finds the proposed addition meets all the criteria of the Special Use by providing enhanced educational services, fully restoring the kindergarten program to the facility, and providing appropriate landscaping and screening for the project. He noted that Staff recommends approval subject to the two conditions listed on page 5 of Staff's report dated January 8, 2018.

Ch. Gassen said that her husband works for Wight and Company, however, due to the nature of this petition, she does not feel that impacts her decision.

Ms. Johnson asked about additional parking spots, and Ms. Leitschuh replied that they already have an excess of parking.

Mr. Kulovany clarified with Mr. Williams that the reason they needed INP-2 zoning was because the 6.7-acre size of the property exceeded the INP-1 area maximum. In further response, Mr. Williams explained that the INP zoning with campus master plans

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includes a variety of institutional uses such as hospitals, stadium seating, parking lots, cell towers, etc.

Ms. Leitschuh added that INP gives the ability to the owner to present various phases of their project to the Village without having to return to the Village for numerous public hearings.

Ch. Gassen called for the Petitioner's presentation.

Amy Fuller of Wight & Company, 2500 N. Frontage Road, Darien, Illinois, made the presentation on behalf of School District 58. She said they intend to add three classrooms to the school to bring the school back to full-day programs, and to return kindergarten facilities to the school. They will provide an ADA accessible entrance, and materials are planned to match the existing brick. They are requesting INP-2 zoning based on the Village's recommendation to comply with the Comprehensive Plan. Construction is anticipated to begin in early March, scheduled for completion at the beginning of the 2018-2019 school year.

Mr. Maurer asked for verification about the materials used, and Ms. Fuller said the materials would match the existing building materials.

Regarding parking, Ms. Fuller said that they comply with the Village's Ordinance calculation based on the number of students. The main striped parking lot is to the south, and during special events there is another area southwest of the site used for overflow parking. She noted that parking is allowed on both Lincoln and on Indianapolis.

Ch. Gassen called upon members of the public who wished to make a comment or ask a question about the petition.

Cindy Gilbert of 240 Lincoln asked how surrounding drainage/water runoff will be affected by the addition, and what measures will be taken to limit construction debris from neighboring properties. They've experienced problems in the past with construction dust and debris in their yards.

Ms. Hartnett of 300 Lincoln said that parents pull in and park at the end of the day where the building is proposed for construction. She is concerned about the safety of the children in the playground area during construction. She also expressed concern that construction traffic will follow the one-way direction during construction. Ms. Hartnett noted an additional concern at night and on weekends with drugs and police activity on the site. There have been occasions when young people have been found on the roof of the building at night. She asked that the property be lit better at night.

There being no further questions or comments, Ch. Gassen closed the public portion of the hearing, and asked the Petitioner to respond to the public comments.

Ms. Fuller showed the plan for construction and where truck traffic will occur. Parent pick-up and drop-off will still take place at the entrance closest to Cumnor Avenue, utilizing the one-way traffic directions. They will maintain the existing site lighting, and

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since the new addition is slightly higher at 18', they don't anticipate anyone climbing up on the roof. They will connect plumbing to the existing sewers toward Lincoln. She then asked their civil engineer to comment on the drainage question.

Mr. Byron Wynn of Wight & Company Land Development Group, 1401 Clark Street, North Aurora, said they are going to create a new sanitary sewer connection along Lincoln, which should not disrupt any sanitary sewer connection to the neighborhood. Roof drainage will be taken to the existing detention basin, and the overall site will maintain the drainage that currently exists. On the east side is an existing detention basin, which will collect stormwater coming from the proposed addition and be sent out to the storm sewer in the street.

Mr. Williams noted that the Village's engineers have reviewed the proposal and there will be a site management permit.

Mr. Maurer asked where the reduction in impervious surface will occur. Mr. Lyons said that they will be adding greenspace around the building and will be removing much of the western driveway's hard surface.

Ms. Leitschuh said if there are any problems or questions from the residents as construction goes on, they should call the Village because there are site management rules that must be followed. The Community Development Department and Code Enforcement Staff go to the sites to assure that all regulations are being met.

Mr. Kulovany stated that this petition seems straightforward.

Mr. Boyle said he hoped this would be a benefit for the community. The engineers will have to meet the regulations.

Ch. Gassen stated that it appears from Staff's report that the requirements for a Special Use have been met. There being no further comments, she called for a Motion.

Ms. Johnson moved with regard to File 17-PLC-0036 that the Plan Commission forward a positive recommendation to the Village Council to approve this request for a Zoning Map Amendment and a Special Use subject to the two conditions listed on page 5 of Staff's report dated January 8, 2018. Mr. Kulovany seconded the Motion.

**AYES: Ms. Johnson, Mr. Kulovany, Mr. Boyle, Mr. Maurer, Ms. Rollins,
Ch. Gassen**

NAYS: None

The Motion passed unanimously.

Mr. Williams said this would be forwarded to the Village Council for their review at their February 6, 2018 meeting.

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17-PLC-0039: A petition seeking approval of a Special Use to construct an automobile dealership. The property is currently zoned B-3, General Services and Highway Business. The property is located on the north side of Ogden Avenue, approximately 385 feet east of Cross Street, commonly known as 2410 Ogden Avenue, Downers Grove, IL (PINs 08-01-303-014, -015, -016, -017). Anas Alkhatib agent of Agri-Pes, LLC, Petitioner; Agri-Pes, LLC, Owner.

Mr. Scott Williams stated that the subject property is zoned B-3 and the petitioner proposes construction of an automobile dealership. He described the surrounding properties, and noted that the site has two access points to Ogden Avenue. The easternmost curb cut along Ogden Avenue will be eliminated. The site has been vacant for about two years and has little existing landscaping. He noted on the site plan the parking areas available on the site. He also pointed out the location of the trash enclosure. The petitioner meets or exceeds all zoning requirements for the location. He noted that there is sufficient space for both vehicle carriers and Fire Department vehicles.

Mr. Williams noted that the property line is almost on the street, and Public Works has requested the granting of a sidewalk easement. The petitioner's landscaping plan meets or exceeds Code requirements as well. The proposed 8500 square foot building is two-story and consists of the actual showroom, as well as rear service bays. He described the elevation as primarily steel, masonry and concrete with a brown colored metal clad paneling, and glazed overhead doors. He said the applicant has submitted a photometric plan with an average foot-candle rating of .1 at centerline of Ogden Avenue. With regard to the Comprehensive Plan Future Land Use Map, the location is shown as Corridor-Commercial.

Mr. Williams said that Staff believes the Special Use criteria have been met as it is an authorized special use, is a redevelopment of a vacant site, it meets the Comprehensive Plan and there have been conditions specific to limiting any potential adverse impact on adjacent properties including test drives in residential areas. Staff recommends approval subject to the seven conditions listed on page 5 of Staff's report dated January 8, 2018.

Ms. Johnson asked about plans for signage. Mr. Williams replied that signage shown will be facing Ogden Avenue.

Mr. Maurer clarified that their only request is for a Special Use for an auto dealership on Ogden Avenue in Downers Grove.

Ch. Gassen called upon the Petitioner to make its presentation.

Paul Chabez, Jr., of Phorma Designs, Inc. of Aurora, said they are taking the existing property that has an abandoned restaurant and replacing that building with a new building, new pavement, and new curb. The dealership will operate between the hours of 10 AM to 8 PM, Monday through Friday, and 11 AM to 6 PM on Saturday and Sunday. The delivery of vehicles will be scheduled on a weekly basis, with all loading and unloading taking place on the dealership lot.

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Ch. Gassen said she thought that automobile dealerships were not allowed to be opened on Sundays. Ms. Leitschuh said she was not sure if there is a law limiting activity on Sunday.

Ms. Johnson noted a different address for Agri-Pes as 857 Willow Lane and asked that the address be corrected. She said she was also surprised to see customer parking in the street yard on their proposed site. Mr. Chabez said they planned to have customer parking in front with automobile display in the back.

Mr. Boyle asked about them reusing the existing building, and Mr. Chabez said the location and condition was not usable. Mr. Boyle asked about the storm drainage.

Mr. Anas Alkhatib replied that the correct address is 857 Willow Lane. As to the existing drainage, the plan is to sheet flow to the southwest corner of the property where there is a culvert that connects to the storm sewer. They will add catch basins to the site, and will keep the same volume and same optimal release.

In response to what kind of automobiles will be sold, Mr. Alkhatib said it is to be high-end used cars with service and detailing available. They will have the site staffed every day.

Ch. Gassen called on the public for comments. There were no comments. She then closed the public portion of the hearing.

Ch. Gassen asked about the sidewalk easement, and Mr. Williams said the Village requested that so they can have access for repair and maintenance.

Mr. Maurer said this proposal looks better than what has been there, and he sees no reason to oppose this. They are asking for a Special Use that's allowed in that Zoning District. He sees it as a benefit.

Mr. Maurer moved with regard to File 17-PLC-0039 that the Plan Commission forward a positive recommendation to the Village Council to approve this request subject to Staff's seven conditions listed on page 5 of Staff's report dated January 8, 2018. Mr. Boyle seconded the Motion.

AYES: Mr. Maurer, Mr. Boyle, Ms. Johnson, Mr. Kulovany, Ms. Rollins, Ch. Gassen

NAYS: None

The Motion passed unanimously.

Mr. Williams said this item would also be forwarded to the Village Council for their review at their February 6, 2018 meeting.

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Ms. Leitschuh stated that there are two items for next month's Plan Commission meeting.

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**Mr. Kulovany moved to adjourn the meeting, seconded by Mr. Maurer.
The Motion carried unanimously by voice vote.**

Ch. Gassen adjourned the meeting at 8:04 PM.

Respectfully submitted,

Tonie Harrington,
Recording Secretary
(transcribed from mp3 recording)

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DEPARTMENT OF COMMUNITY DEVELOPMENT MEMO

To: Plan Commission
From: Rebecca Leitschuh, Senior Planner
Subject: **17-PLC-0014: PUD, Special Use, and Alley Vacation**
2125 Ogden Avenue
Date: January 31, 2018

The petitioner has requested to continue the Planned Unit Development, Special Use, and Alley Vacation petition. A similar development proposal was before you in November of 2017, with a unanimous recommendation for approval before Village Council on November 21st. Following the Village Council meeting, the applicant decided to propose minor modifications to their proposal, including petitioning for a Planned Unit Development. The changes warrant returning to Plan Commission to review the building modifications and the revised entitlement process.

Staff is recommending that the Plan Commission grant the petitioner's request to continue the public hearing to the March 5, 2018 Plan Commission meeting.



**VILLAGE OF DOWNERS GROVE
REPORT FOR THE PLAN COMMISSION
FEBRUARY 5, 2018 AGENDA**

SUBJECT:	TYPE:	SUBMITTED BY:
17-PLC-0041 2001 63 rd Street	PUD Amendment, Special Use and Plat of Subdivision	Rebecca Leitschuh, AICP Senior Planner

REQUEST

The petitioner is requesting approval for an amendment to Planned Unit Development #1 to allow the construction of a new Walgreens store, a Special Use to allow a drive-through pharmacy and a Plat of Subdivision at 2001 63rd Street.

NOTICE

The application has been filed in conformance with applicable procedural and public notice requirements.

GENERAL INFORMATION

OWNER & APPLICANT: FL Cedar, LLC
477 Elm Place
Highland Park, IL 60035

PROPERTY INFORMATION

EXISTING ZONING: B-2/PUD, General Retail Business/Planned Unit Development
EXISTING LAND USE: Shopping Center
PROPERTY SIZE: 69,753 sq ft (1.6 acres)
PINS: 08-24-202-008 & -009

SURROUNDING ZONING AND LAND USES

	ZONING	FUTURE LAND USE
NORTH:	R-4, Single Family Unincorporated DuPage County	Single-Family Attached
SOUTH:	R-6, Residential Apartment/Condo 6	Multi-Family Residential
EAST:	R-3, Residential Detached House 3	Single-Family Attached, Neighborhood Commercial
WEST:	B-2, General Retail Business	Mixed Use

ANALYSIS

SUBMITTALS

This report is based on the following documents, which are on file with the Department of Community Development:

1. Project Narrative
2. Plat of Survey
3. Architectural Plans
4. Engineering Plans
5. Landscape Plan
6. Photometric Plan
7. Elevations and Renderings
8. Traffic Impact Study
9. Neighborhood Meeting Summary Report
10. Plat of Subdivision

PROJECT DESCRIPTION

The applicant is proposing to construct a Walgreens pharmacy at 2001 63rd Street. A similar proposal (16-PLC-0062) was approved in August 2017, although the siting, orientation, size, and architectural design of the building has been revised. The subject area involves 1.6 acres of the 18.86 acre shopping center property, located at the southwest corner of 63rd Street and Woodward Avenue. The property is zoned B-2/PUD, General Retail Business/Planned Unit Development and encompasses existing Planned Unit Development #1. The petitioner is requesting:

- A PUD Amendment to permit the construction of a Walgreens
- A Special Use for the construction of a drive-through
- A Plat of Subdivision to create the Walgreens out-lot and a second out-lot for future development

The petitioner is proposing to build a new 10,500-square-foot Walgreens building at the southwest corner of the intersection of Woodward Avenue and 63rd Street. The proposed development would include the demolition of a vacant restaurant building at this location. The project site for this new building is approximately 1.08 acres and will include a convenience store and drive-through pharmacy with 43 parking spaces.

The petitioner is also proposing to create an additional out-lot (0.52 acres) for future commercial use through the final Plat of Subdivision. There are no immediate plans to develop the out-lot. In the interim, the existing pavement will be removed, and the entire lot will be seeded per the landscape requirements, reducing the shopping center's overall impervious surface.

The drive-through facility will be located on the south side of the building with one-way only traffic allowed with appropriate signage to direct traffic. An ADA accessible path is proposed from the corner of the 63rd/Woodward intersection to the entrance of the building. Parking is provided on the western side of the building, with four rows of parking and two full-access drive aisles, and exceeds the requirements of parking per the Zoning Ordinance. There will be two ADA accessible parking spaces adjacent to the building's main entrance as required. The trash compactor, transformer, and tote enclosure are located on the east side of the building, fully enclosed by a wall matching the style of the building.

The petitioner is proposing landscaping in conformance with the Village requirements. Landscaping is provided on all four sides of the property. A total of 34 shade trees are intermixed with shrubs and ornamental grasses around the perimeter, the interior parking lot islands, and the drive aisles. Parking lot and site lighting complies with Village requirements.

The previously approved Walgreens was clad principally in an exterior insulation and finish system (EIFS). The color scheme was principally gray with some red accents. Based on previous discussions, the petitioner has revised the exterior design of the building to minimize the use of EIFS, use fiber cement

architectural panels as the principal material and provide a variety of colors and textures. The facades are broken up by a light brown (Tuscan) modern block face, a white (Chantilly Lace) smaller brick face, and a horizontal wood panel (Cedar), all made of fiber cement board. These materials are further varied with the use of a white EIFS overhang that runs along the roof line of the building on its street facing walls and main western entrance. The main entrance incorporates all of these elements, with the addition of windows and a white horizontal band, breaking the light brown block face. A sign is featured over the entrance.

The 63rd Street facing (north) elevation wraps the corners with the wood panel elements, and breaks up the expanse with windows on both corners. A vertical pier made of the small white brick panel anchors the main corner. A second building sign is proposed on the north elevation.

The Woodward Avenue (east) elevation continues to wrap the corner with wood panels, windows, and the white EIFS overhang. Staff requests a condition be made to extend the EIFS design element across the entire length of the eastern wall so as to bring greater design detail to the east elevation. A vertical pier, identical to the one on the western elevation, intersects the horizontal planes. The dumpster enclosure, while in a street yard, is designed to complement the building, incorporating the same fiber cement panel system.

The rear (south) wall has a canopy over the drive-through window, surrounded by the wood panel design. The horizontal white accent band continues from the other elevations. Roof top mechanical units will be screened from the public right-of-ways. All proposed signage for Walgreens complies with the square-footage requirements of the sign ordinance, including a single tenant monument sign at the northwest corner. The side interior wall sign, although not a location permitted by-right, is supported by staff since it identifies the main entrance.

A reference table is provided below with a quick comparison of the previously approved petition (16-PLC-0062) and the revised submittal.

Walgreens	16-PLC-0062	17-PLC-0041
Exterior Finish Materials	Gray EIFS with red accents	Nichiha fiber cement board (cedar, white brick, light brown block), EIFS overhang
Building Location	Western side of lot	Northeast corner
Building Size	14,500 sq ft	10,500 sq ft
Parking Spaces (required/provided)	51/66	37/43
Building Height	29.3 ft	20 ft
Shopping Center Improvements	Yes	Yes
63 rd Street Improvements	Yes	Yes
Subdivision (2 original lots)	Reconfigured	Addition of 1 out-lot

In the previous submittal to Plan Commission (16-PLC-0062), Walgreens final approval and occupancy was contingent on making significant improvements to the entire shopping center. The petitioner has started to implement some of these improvements, and is in for permit review of the at-grade site work. The occupancy of Walgreens will still be contingent on implementing all of the identified site and building façade improvements under 16-PLC-0062. These improvements include the following:

- Removal of the existing 63rd Street dual access points and replacement with a single three-quarter access point
- Façade renovations for all shopping center buildings including new EIFS facades with corner treatments, accent bands and new column enclosures.
- Installation of new curbed landscape islands within the front parking lot
- Repaired parking lot and drive aisle along 63rd Street
- Repair of rear access drive and replacement of speed bumps within the rear access drive
- Removal of rear southernmost access point to Belmont Road
- Removal of excess pavement in southwest corner of the shopping center
- Repair of low lying area in the rear of the center which leads to ponding water

COMPLIANCE WITH THE COMPREHENSIVE PLAN

The Comprehensive Plan's Future Land Use Map designates this property as Mixed Use, and it is identified as the only catalyst site within the 63rd Street focus area plan. As a Mixed Use property, the plan recommends "a mix of land uses within a contiguous geographic boundary" serving more than one purpose. The 63rd Street Focus Area Plan notes that the Village should encourage commercial expansion at key intersections where existing commercial uses exist and where it is necessary to improve their vitality. The plan also identifies the enhancement of access and visibility within nearby parcels, and to connect nearby residential areas to shopping and services through pedestrian and bicycle access. In addition, commercial developments should attempt to reduce the urban heat island effect through shading and the use of light-colored building materials; of which both elements are incorporated in this proposal.

As a catalyst site, the plan notes that Meadowbrook Shopping Center should include a mix of uses, and that the site could be redeveloped to include both residential and commercial uses. The plan does not mandate that both residential and commercial uses be a part of a redevelopment. The plan merely identifies the potential for a mix of residential and commercial if the property is no longer viable as a commercial center. The Commercial and Office Area Goal #2 includes the objectives to promote the "... redevelopment of the Meadowbrook Mall and other outdated shopping centers" and to identify and work with "...underperforming and underutilized" sites. The goal encourages the Village to enhance the economic vitality, productivity, appearance and function of commercial corridors including 63rd Street. Additionally, the 63rd Street redevelopment concept graphic identifies commercial out-lots along 63rd Street, supporting the creation of two out-lots.

The proposed redevelopment is consistent with the goals of the Comprehensive Plan.

COMPLIANCE WITH ZONING ORDINANCE

The property is zoned B-2/PUD, General Retail Business/Planned Unit Development, established in the 1970s.

The bulk requirements of the proposed Walgreens development in the B-2/PUD zoning district are summarized in the following table:

Zoning Requirements

2001 63rd Street (Lot 3)	Required	Proposed
North Setback (Street Yard – 63 rd Street) - Building	25 ft	25.32 ft
East Setback (Street Yard – Woodward Avenue) - Building	25 ft	51.5 ft
South Setback (Rear Yard) - Building	n/a	31.5 ft
West Setback (Side Yard) - Building	n/a	133.5 ft
West Setback - Parking	n/a	3.5 ft
North Setback - Parking	25 ft	27.5 ft
South Setback – Parking	n/a	8 ft
Landscaped Open Space	4,714 sf (10%)	12,256 sf (26%)
Street yard Landscaped Open Space	2,357 sf (5%)	10,163 sf (22%)
Floor Area Ratio	0.75 (max)	0.22
Building Height	35 ft (max)	20 ft
Loading Setback	50 ft	67 ft
Parking Spaces	37	43
Drive-through Stacking	3	3
Drive-through Setback	25 ft	20.5 ft

The proposed Walgreens development is consistent with the requirements of the Zoning Ordinance, excepting the drive-through setback minimum distance. However, staff finds the setback reduction of 4.5 feet is acceptable considering the proposed location of the drive-through further improves on-site circulation and better screens the service. The proposed Walgreens and site improvements will not negatively impact the amount of remaining parking for the rest of the shopping center. The applicant’s proposal is consistent with the Village’s Zoning Ordinance.

COMPLIANCE WITH SUBDIVISION ORDINANCE

The petitioner is proposing to create two new lots out of the existing Lot 2 in the shopping center. Lot 3 will be used for Walgreens and Lot 4 is the additional out-lot. The revised Lot 2, new Lot 3 and Lot 4 will meet the minimum lot width and lot area requirements outlined in Section 20.301 of the Village’s Subdivision Ordinance. The other two existing lots (Lot 1 and Lot 5) will remain the same size.

Meadowbrook Subdivision	Lot Width (req. 100 ft.)	Lot Depth (req. 140 ft.)	Lot Area (req. 10, 500 sq. ft.)
Lot 2	450.16 ft	475.81 ft	225,238 sq. ft. (5.17 ac)
Lot 3	163.41 ft	285 ft	47,138 sq. ft. (1.08 ac)
Lot 4	140.37 ft	163.41 ft	22,615 sq. ft. (0.52 ac)

The petitioner is providing a cross-access easement that connects the new 63rd Street access point to the northernmost access points along Belmont Road and Woodward Avenue. The easement is further extended

to include the drive aisle in front of the Meadowbrook Shopping Center, and also wraps around Lot 5 to the south. This will ensure perpetual access through a non-exclusive easement for the benefit of all lots in the subdivision. Additionally, the petitioner is providing the required five-foot wide public utility and drainage easements along the side lot lines and the ten-foot wide public utility and drainage easements along the rear lot lines for Lots 3 and 4.

ENGINEERING/PUBLIC IMPROVEMENTS

There is a net decrease in the impervious area and therefore new stormwater detention is not required. The drainage for the site will tie into the existing stormwater system for the shopping center. The petitioner will be required to meet all Village engineering standards and comply with all applicable codes when formally submitting for a permit.

The petitioner is proposing to eliminate the dual full-access points onto 63rd Street and replace them with a single three-quarters access point. The two current 63rd Street access points are approximately 30 feet from each other. These two access points can create confusion and conflict points for both drivers entering and exiting the site and also for pedestrians walking along the 63rd Street sidewalk. In conjunction with DuPage County, the petitioner has proposed to combine these access points into a single access point. The single access will allow both eastbound and westbound 63rd Street traffic to enter the site, but will limit the exit point to a right-turn (eastbound) only. DuPage County is requiring the petitioner to dedicate land along 63rd Street to extend the turn lane going east. At time of permit, the petitioner will have to verify the location and elevation of an existing Village water main. As a result of the right-turn lane extension, the petitioner may have to relocate or protect the existing water system.

TRAFFIC

A traffic impact study for the proposed development was completed by the petitioner. The study examined the existing 63rd Street and Woodward Avenue traffic conditions and the future conditions based on the proposed development.

The study found that the proposed new store will generate new trips during the weekday evening and Saturday midday peak hours; however, this will not have a detrimental effect on the shopping center or surrounding properties given the multiple access points on the site. The total existing traffic on 63rd Street is over 27,000 vehicles per day, which will be increased by less than 2% with the proposed development. Also noted in the study is the significant number of pass-by trips. Pass-by trips are vehicles that are already using 63rd Street but will now stop at the proposed development and do not represent an increase in traffic.

The study also considers the conversion of the dual full-access points on 63rd Street to a three-quarter access, prohibiting left turns on 63rd Street, as an improvement and reduction in traffic conflict points for the property. Removing these conflict points will reduce the opportunity for crashes. The proposed development's impact on the geometry of 63rd Street should provide a safety benefit.

PUBLIC SAFETY REQUIREMENTS

The Fire Prevention Division reviewed the proposed development and determined that sufficient access to and around the site is provided for emergency vehicles. The site layout permits Fire Department apparatus the opportunity to enter and exit the site from both 63rd Street and Woodward Avenue. The building will be required to include a fire alarm and sprinkler system that meet the Village's code requirements. A fire hydrant will be required to be installed as part of this approval on the vacant lot for future use.

NEIGHBORHOOD COMMENT

Notice was provided to all property owners 250 feet or less from the property in addition to posting public

hearing notice signs and publishing the legal notice in the *Downers Grove Suburban Life*. No public comments have been received by staff.

The petitioner held a neighborhood meeting on November 28, 2017 with four current tenants in attendance. Questions were asked about construction timing, and shopping center signage and improvements. The applicant responded to each of these topics during the meeting and has provided a summary of the meeting that is attached.

FINDINGS OF FACT

The petitioner is requesting a Planned Unit Development Amendment, a Special Use and a Plat of Subdivision to construct a new retail and pharmacy store at 2001 63rd Street. Staff finds that the proposal meets the standards for granting a Planned Unit Development Amendment, a Special Use and Plat of Subdivision as outlined below:

Section 28.12.040.C.6 Review and Approval Criteria

The decision to amend the zoning map to approve a PUD development plan and to establish a PUD overlay district are matters of legislative discretion that are not controlled by any single standard. In making recommendations and decisions regarding approval of planned unit developments, review and decision-making bodies must consider at least the following factors:

a. The zoning map amendment review and approval criteria of Sec. 12.030.1.

As previously noted, the shopping center was approved as a Planned Development in the 1970s. Section 4.030.C of the Zoning Ordinance, adopted in 2014, notes that all previously approved Planned Developments were reclassified as Planned Unit Developments. As such, a rezoning is not required and this standard does not apply.

b. Whether the proposed PUD development plan and map amendment would be consistent with the comprehensive plan and any other adopted plans for the subject area.

The proposed project is consistent with the Comprehensive Plan. The plan identifies this area as *Mixed Use*. This property is an existing shopping center and the proposed use will be compatible with the other uses. The site is well suited to accommodate a drive-through pharmacy. The proposed development is consistent with the policy recommendation that mixed use areas provide a variety of land uses within a pedestrian accessible neighborhood. The proposed improvements will enhance the economic vitality productivity, appearance and function of the shopping center as identified in Commercial and Office Area Goal #2.

The proposed project is consistent with the Comprehensive Plan and the 63rd Street focus area plan. The project is designed in a manner that is compatible with surrounding land uses. The proposed Walgreens and both the building and site improvements to the shopping center will improve the vitality of the center. The proposed removal of two access points onto 63rd Street and the installation of a single three-quarters access point will enhance access to 63rd Street while improving safety. A second new out-lot is proposed, that is consistent with the redevelopment concept sketch while also revitalizing an aged shopping center.

The Focus Ara Plan notes a mix of uses could be provided if the property is no longer viable as a commercial center. The improvements proposed by the property owner show that the owner believes a redevelopment of the commercial space is feasible and will lead to enhanced economic vitality in the center.

This standard has been met.

- c. *Whether PUD development plan complies with the PUD overlay district provisions of Sec. 4.030.***
The proposed project meets several of the PUD overlay district provisions and objectives as found in Section 4.030 of the Zoning Ordinance. The PUD is consistent with and helps advance the goals of the Comprehensive Plan. The development also meets the PUD overlay district provisions by providing a high quality building that is compatible with other developments along 63rd Street while providing attractive, high-quality landscaping for the Walgreens site and numerous upgrades to the property. Improvements have been proposed to improve motorized and non-motorized travel on-site such as a reduction of dual access points along 63rd Street to a single access point which increases safety along the public right-of-way and an accessible route connecting the front entrance of the building to the sidewalk. This standard has been met.
- d. *Whether the proposed development will result in public benefits that are greater than or at least equal to those that would have resulted from development under conventional zoning regulations.***
The proposed development will result in a new convenience store and pharmacy for the neighborhood, and an additional out-lot for future commercial development, in compliance with the Comprehensive Plan. The proposed development meets many objectives of the Comprehensive Plan and furthers the vision of the Village to improve 63rd Street. The new building will enhance the aesthetics of the shopping center and 63rd Street. The public benefits include the replacement of dual access points to 63rd Street with a single three-quarters access point. This will eliminate conflicts between vehicles and vehicles and pedestrians. The building and site improvements will enhance the vitality of the shopping center and this section of 63rd Street. This standard has been met.
- e. *Whether appropriate terms and conditions have been imposed on the approval to protect the interests of surrounding property owners and residents, existing and future residents of the PUD and the general public.***
There are several conditions noted below that will protect the interests of the surrounding neighborhood and the general public. The conditions below are being requested to ensure that the proposed development satisfies all applicable codes and requirements. The project will advance many goals and objective laid out in the current and updated Comprehensive Plan and the conditions listed below will ensure that these goals and objectives are met. Several improvements provided by the petitioner for the existing shopping center (through 16-PLC-0062) will enhance the overall property and will be an improvement for the neighborhood. This standard has been met.

Section 28.12.050.H Approval Criteria

No special use may be recommended for approval or approved unless the respective review or decision-making body determines that the proposed special use is constituent with and in substantial compliance with all Village Council policies and plans and that the applicant has presented evidence to support each of the following conclusions:

- 1. That the proposed use is expressly authorized as a Special Use in the district in which it is to be located;***
The property is located in the B-2/PUD, General Retail Business/Planned Unit Development zoning district. Under Section 5.010 of the Zoning Ordinance, a drive-through facility is listed as an allowable Special Use in the B-2 zoning district. This standard has been met.
- 2. That the proposed use at the proposed location is necessary or desirable to provide a service or a facility that is in the interest of public convenience and will contribute to the general welfare of the neighborhood or community.***
The proposed drive through pharmacy is a desirable service to the community and will contribute to the general welfare of the Village. The drive-through pharmacy provides a convenient service to the

community. The development will cater to the local customers as desired in the existing Comprehensive Plan and will meet many goals and objectives outlined in both the current and updated Comprehensive Plan. This standard has been met.

3. *That the proposed use will not, in the particular case, be detrimental to the health, safety or general welfare of persons residing or working in the vicinity or be injurious to property values or improvements in the vicinity.*

The proposed drive-through will not be detrimental to the health, safety or general welfare of persons residing in or working in the vicinity and will not be injurious to property values or improvements in the vicinity. The drive-through is located along the southern wall, away from residential properties and from adjacent public sidewalks, with ample landscaping to screen properties across the street. The location of the building will lead to other improvements including the elimination of dual access points onto 63rd Street which will create a safer driving and walking environment in this area. This standard is met.

Section 20.301 – Plat of Subdivision

The proposed subdivision meets the minimum lot area and width requirements of Sections 20.301 of the Subdivision Ordinance.

RECOMMENDATIONS

The proposed Planned Unit Development Amendment, Special Use for a drive through and the Plat of Subdivision for the new development in Meadowbrook Shopping Center at 2001 63rd Street is consistent with the current and updated Comprehensive Plans, the Zoning Ordinance, the Subdivision Ordinance and surrounding zoning and land use classifications. Based on the findings listed above, staff recommends the Plan Commission recommend the Village Council **approve** the requested Planned Unit Development Amendment, Special Use and Plat of Subdivision as requested in case 17-PLC-0041 subject to the following conditions:

1. The Planned Unit Development, Special Use and Plat of Subdivision shall substantially conform to the staff report; and drawings prepared by Manhard Consulting Ltd, dated 12/20/2017 and resubmitted on 01/18/2018, except as such plans may be modified to conform to the Village codes and ordinances.
2. The site improvement work for the property must be completed per the Site Improvement Exhibit, dated 11/28/16, revised plan dated 01/19/2017, and approved by Village Council in August 2017 prior to the issuance of the Certificate of Occupancy for Walgreens.
3. The Walgreens building shall be equipped with an automatic suppression system and an automatic and manual fire alarm system.
4. A fire hydrant shall be installed, including water/fire service line on the vacant out-lot for future use.
5. A separate sign permit will be required prior to installation of any wall or monument sign.
6. The white exterior insulation and finish system (EIFS) shall be extended across the entire length of the roof-line facing Woodward Avenue.
7. The EIFS on the building shall be maintained in accordance with the Village's currently adopted edition of the International Property Maintenance Code.
8. No building permits can be issued until the Final Plat of Subdivision is recorded.
9. A pedestrian connection shall be provided from Woodward Avenue across the southern property line of Lot 3.

10. The petitioner shall provide elevations of new pavement over the water main in response to the right-turn lane extension per DuPage County. Petitioner shall protect and/or relocate existing water system if necessary.

Staff Report Approved By:



Stanley J. Popovich, AICP
Director of Community Development

SP; rl
-att

December 20, 2017
Revised: January 18, 2018

Mr. Stan Popovich
Village of Downers Grove
801 Burlington Avenue
Downers Grove, IL 60515

Re: Project Summary/Narrative
Proposed Walgreens
SW Corner 63 Street and Woodward Avenue

Dear Mr. Popovich:

Please accept this letter as a request by FL Cedar, LLC (Owner) for approval of the application for Special Use for a Drive-Thru, Amendment to Existing PUD, and a Plat of Subdivision.

The original Petition for Plan Commission of this PUD Amendment was submitted to the Village of Downers Grove on November 29, 2016 under Village Project No. 16-PLC-0062. Upon Village review, the PUD Amendment was approved by the Village Council (Ordinance No. 5640 and 5641) on August 8, 2017. Since that time, the end user has chosen to modify the total square footage of the proposed Walgreens facility. As a result of this change, the site plan and parking configuration has been updated accordingly. Furthermore, the proposed lot configuration has been adjusted based on the revised site plan. A convenience store, pharmacy, and drive thru pharmacy will remain part of the new Petition for Plan Commission.

The project site is located at the southwest corner of 63rd Street and Woodward Avenue. The project site is approximately 1.6 acres, and it is currently occupied by an existing building. This existing building was formerly used as a restaurant. The site has frontage along the 63rd Street to the north and Woodward Avenue to the east. The south and west boundaries are abutting the existing shopping center parking lot and drive aisles. The project site has access to 63rd Street and Woodward Avenue via the existing shopping center. The site is currently zoned B-2 PD (General Retail Business Planned Development).

The Owner proposes to demolish the existing unoccupied building and construct a new 10,500 SF Walgreens store including a pharmacy drive-thru, 41 parking spaces, and associated landscaping. The Walgreens will consist of a convenience store, pharmacy, and drive thru pharmacy. The store will employ approximately 35 part-time and full-time employees that will work on various assigned shifts. The hours of operation are proposed to be 8:00 am to 10:00 pm for the store and 8:00 am to 8:00 pm for the pharmacy and the drive-thru. The Owner is proposing to combine the existing dual access points off of 63rd Street into one access point. Coordination with DuPage County Division of Transportation is ongoing.

In addition to the proposed Walgreens, the Owner is also coordinating with the Village on providing numerous upgrades to the existing shopping center including existing façade improvements, asphalt repairs to the east-west drive aisle and north parking lot, asphalt repairs and traffic calming measures to the rear drive aisle in the southern portion of the shopping center, and landscaping improvements to the shopping center parking lot. The referenced overall shopping center improvements are currently under review by the Village of Downers Grove Staff, and an ordinance approving the Amendment to Planned Unit Development (P.U.D.) was approved by the Village Council (Ordinance No. 5641) on August 8, 2017.

The proposed Walgreens is a permitted use by right in the B-2 General Retail Business district. The drive-thru requires a Special Use approval.

For additional detailed information, please also refer to the submitted plans titled Proposed Walgreens, dated 12/20/2017 (Revised 01/18/18) prepared by Manhard Consulting, as well as plans, elevations and renderings dated 12/20/2017 (Revised 01/18/18) prepared by Camburas & Theodore, Ltd.

The requested Special Use Approval, Plat of Subdivision, and Amendment to Existing PUD are in conformance with the Village Municipal Code standards and the following is the evidence to support these request:

Request for Special Use Approval Criteria (Section 28.12.050.H)

No special use may be recommended for approval or approved unless the respective review or decision-making body determines that the proposed special use is consistent with and in substantial compliance with all village council policies and plans and that the applicant has presented evidence to support each of the following conclusions:

1. that the proposed use is expressly authorized as a special use in the district in which it is to be located; **The proposed use is expressly authorized as a special use in the B-2 General Retail Business district.**
2. that the proposed use at the proposed location is necessary or desirable to provide a service or a facility that is in the interest of public convenience and will contribute to the general welfare of the neighborhood or community; **The proposed use at the proposed location is necessary and desirable as it provides a convenience to the community as well as additional safety for customers of the pharmacy. Customers, as a result of the drive-thru facility, are not required to park, exit their vehicle, and walk into the store in order to get a prescription filled.**
3. that the proposed use will not, in the particular case, be detrimental to the health, safety, or general welfare of persons residing or working in the vicinity or be injurious to property values or improvements in the vicinity. **The proposed use will be a benefit to the health, safety, and general welfare of the community as the drive thru provides for additional safety as customers are not required to park, exit their vehicle, and walk into the store in order to get a prescription filled. The proposed use will not be injurious to property values or improvements in the vicinity as there will be a newly constructed building, new parking lot, new landscaping, and a new ADA accessible route from the building to the adjacent roadway right-of-way. These proposed improvements will be in conformance with the Village Municipal Code and standards.**

Request Amendment to Existing PUD (Section 28.12.040.C.6)

The decision to amend the zoning map to approve a PUD development plan and to establish a PUD overlay district are matters of legislative discretion that are not controlled by any single standard. In making recommendations and decisions regarding approval of planned unit developments, review and decision-making bodies must consider at least the following factors:

- a. the zoning map amendment review and approval criteria of Sec. 12.030I in the case of new Planned Unit Development proposals; **This is an existing PUD.**
- b. whether the proposed PUD development plan and map amendment would be consistent with the comprehensive plan and any other adopted plans for the subject area; **The PUD Development Plan is consistent with the comprehensive plan as this site is located within the Corridor Commercial area.**
- c. whether PUD development plan complies with the PUD overlay district provisions of Sec. 4.030; **The PUD Development plan is in conformance with the vision and goals of the comprehensive plan.**

d. whether the proposed development will result in public benefits that are greater than or at least equal to those that would have resulted from development under conventional zoning regulations; and **The public benefits are greater than those that would have resulted from the conventional zoning because of the added convenience to the community as well as additional safety for customers who are not required to park, exit their vehicle, and walk into the store in order to get a prescription filled.**

e. whether appropriate terms and conditions have been imposed on the approval to protect the interests of surrounding property owners and residents, existing and future residents of the PUD and the general public. **The proposed use is unobtrusive and does not create noise issues. The proposed use is buffered effectively by not only the natural terrain, but by both 63rd Street and Woodward Avenue to the north and east respectively.**

Planned Unit Development Overlay District Provisions (Section 4.030.A.2)

Different types of PUDs will achieve different planning goals. In general, however, PUDs should include elements that further some or all of the following objectives:

a. implementation of and consistency with the comprehensive plan and other relevant plans and policies; **The subject site is located in, and consistent with, the Corridor Commercial area.**

b. flexibility and creativity in responding to changing social, economic and market conditions allowing greater public benefits than could be achieved using conventional zoning and development regulations; **Not applicable**

c. efficient and economical provision of public facilities and services; **Not applicable**

d. variety in housing types and sizes to accommodate households of all ages, sizes, incomes and lifestyle choices; **Not applicable**

e. compact, mixed-use development patterns where residential, commercial, civic and open spaces are located in close proximity to one another; **Not applicable**

f. a coordinated transportation system that includes an inter-connected hierarchy of facilities for motorized and non-motorized travel; **The existing subject site includes cross access drive aisles for motorized travel that connect Woodward Avenue and Belmont Road without having to utilize 63rd Street. The proposed development will utilize these existing cross access drives. Additionally, the reduction of dual access points along 63rd Street to a single access point is an increase in the safety of the public. Non-motorized travel will be accommodated by the proposed accessible route from the front of the proposed building to the existing sidewalks at the southwest corner of 63rd Street and Woodward Avenue.**

g. high-quality buildings and improvements that are compatible with surrounding areas, as determined by their arrangement, massing, form, character and landscaping; **In addition to the proposed Walgreens, the Owner is also coordinating with the Village on providing numerous upgrades to the existing shopping center including existing façade improvements, asphalt repairs to the east-west drive aisle and the north parking lot area, asphalt repairs and traffic calming measures to the rear drive aisle in the southern portion of the shopping center, and additional landscaping in the existing shopping center parking lot. The referenced overall shopping center improvements are currently under review by the Village of Downers Grove.**

h. the protection and enhancement of open space amenities and natural resource features; **In addition to the proposed Walgreens landscaping and open space, the Owner is also coordinating with the Village on providing numerous landscaping improvements within the existing shopping center parking lot. The referenced overall shopping center improvements are currently under review by the Village of Downers Grove.**

i. the incorporation of sustainable development features including green infrastructure practices in landscapes and parking area, to maximize the aesthetic and water quality benefits of best practices in stormwater management; and **The proposed Walgreens landscaping and open space meets, and in some cases exceeds, the Village Municipal Code and standards.**

j. attractive, high-quality landscaping, lighting, architecture and signage, including the use of native landscaping that reflects the unique character of the village and the surrounding area. **The proposed Walgreens landscaping, open space, lighting, and signage meets the Village Municipal Code and standards and does reflect the unique character of the village. The numerous upgrades to the existing shopping center façade, parking lot, and landscaping will also provide a significant enhancement to the surrounding area. The referenced overall shopping center improvements are currently under review by the Village of Downers Grove.**

Developer's Statement of Intent Section 4.030.D

The proposed project is an amendment to an existing PUD and consists of the demolition of an existing building (former restaurant) and construction of a new 10,500 SF Walgreens store including a pharmacy drive-thru, 41 parking spaces, new site lighting, and landscaping. The project also includes numerous upgrades to the existing shopping center including existing façade improvements, asphalt repairs to the east-west drive aisle and the north parking lot area, asphalt repairs and traffic calming measures to the rear drive aisle in the southern portion of the shopping center, and additional landscaping in the shopping center parking lot.

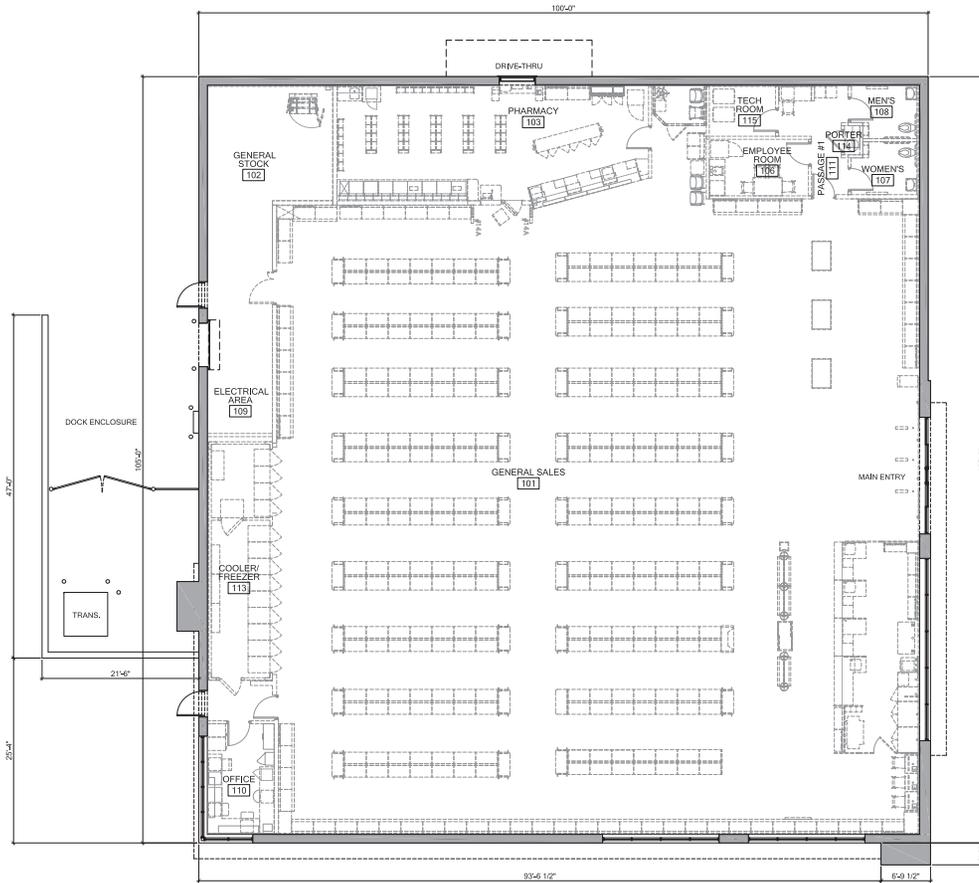
There are many benefits to the existing shopping center, the surrounding area, and the community. Access to over the counter and prescription medication is a rudimentary need and a necessity to the community. The proposed drive-thru will be a benefit to the health, safety, and general welfare of the community as the drive thru provides for additional safety as customers are not required to park, exit their vehicle, and walk into the store in order to get a prescription filled. The reduction of dual access points along 63rd Street to a single access point is an increase in the safety of the public. The new ADA accessible route from the building to the adjacent roadway right-of-way will provide a benefit to the pedestrians along Woodward Avenue and 63rd Street. The numerous upgrades to the existing shopping center will enhance the overall appearance of the shopping center while also making the shopping center much better and safer for the community.

We appreciate the opportunity to present this project to you for approval. If you have any questions or require additional clarification, please do not hesitate to contact us at 773-571-4199.

Sincerely,



Perrine Knight
FL Cedar, LLC



1 GENERAL FLOOR PLAN
SCALE: 1/8" = 1'-0"



DATE	12-20-17
BY	AS
CHECKED	AS NOTED
SCALE	AS NOTED
PROJECT	63RD STREET AND WOODWARD AVENUE
SHEET	A-111

Manhard CONSULTING LTD.
 1000 Lakeshore Blvd. W. Suite 1000
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 Email: info@cameronthorp.com

63RD STREET AND WOODWARD AVENUE
 VILLAGE OF DOWNERS GROVE, ILLINOIS
 FLOOR PLAN

PROJECT	63RD STREET AND WOODWARD AVENUE
DATE	12-20-17
BY	AS
CHECKED	AS NOTED
SCALE	AS NOTED

SHEET
A-111

- SITE DIMENSIONAL AND PAVING NOTES:**
- ALL DIMENSIONS ARE FACE OF CURB TO FACE OF CURB OR BUILDING FOUNDATION UNLESS NOTED OTHERWISE.
 - ALL PROPOSED CURB AND GUTTER SHALL BE B6.12 UNLESS OTHERWISE NOTED.
 - ALL CURB RADI SHALL BE 3' MEASURED TO FACE OF CURB UNLESS NOTED OTHERWISE.
 - THE ALL PROPOSED CURB AND GUTTER TO EXISTING CURB AND GUTTER WITH 2" x 6 BARS x 18" LONG DOWELED INTO EXISTING CURB.
 - BUILDING DIMENSIONS AND ADJACENT PARKING HAVE BEEN PREPARED BASED UPON ARCHITECTURAL INFORMATION CURRENT AT THE DATE OF THIS DRAWING. SUBSEQUENT ARCHITECTURAL CHANGES MAY EXIST. THEREFORE CONTRACTOR SHALL REFER TO ARCHITECTURAL DRAWINGS FOR BUILDING DIMENSIONS AND NOTIFY THE ARCHITECT AND ENGINEER OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION. BUILDING DIMENSIONS SHOWN SHOULD NOT BE USED FOR CONSTRUCTION LAYOUT OF BUILDING.

- IMPROVEMENTS ADJACENT TO BUILDING, IF SHOWN, SUCH AS TRUCK DOCK, RETAINING WALLS, SIDEWALKS, CURBING, FENCES, CANOPIES, RAMPS, HANDICAP ACCESS, PLANTERS, DUMPSTERS, AND TRANSFORMERS ETC. HAVE BEEN SHOWN FOR APPROXIMATE LOCATION ONLY. REFER TO ARCHITECTURAL PLANS FOR EXACT LOCATIONS, SPECIFICATIONS AND DETAILS.
- LOCATION OF PRIVATE SIDEWALKS SHALL BE COORDINATED WITH PROPOSED DRIVEWAY. CONTRACTOR TO VERIFY ACTUAL BUILDING PLAN LOCATIONS WITH ARCHITECT/DEVELOPER PRIOR TO CONSTRUCTING THE SIDEWALKS.
- ALL ROADWAY AND PARKING LOT SIGNAGE, STRIPING, SYMBOLS, ETC. SHALL BE IN ACCORDANCE WITH LATEST JURISDICTIONAL GOVERNMENTAL ENTITY DETAILS.
- SOME EXISTING ITEMS TO BE REMOVED HAVE BEEN DELETED FROM THIS PLAN FOR CLARITY. SEE DEMOLITION PLAN FOR ITEMS DELETED.
- DEPRESS CURB & GUTTER AT ALL SIDEWALK AND PATH LOCATIONS FOR HANDICAP ACCESS PER FEDERAL AND STATE STANDARDS.
- THE CONTRACTOR SHALL CONTACT JULIE L. (1-800-892-0123) PRIOR TO ANY WORK TO LOCATE UTILITIES AND SHALL CONTACT THE OWNER SHOULD UTILITIES APPEAR TO BE IN CONFLICT WITH THE PROPOSED IMPROVEMENT.
- ALL TRAFFIC SIGNS SHALL BE INSTALLED AT 7' HIGH, MEASURED FROM THE GROUND ELEVATION TO THE BOTTOM OF THE SIGN.
- THE STOP BAR AT 63RD STREET DRIVEWAY SHALL BE INSTALLED WITH A CONCRETE BASE.
- TRAFFIC SIGNS INSTALLED WITHIN THE COUNTY R.O.W. SHALL NOT BE INSTALLED WITH A CONCRETE BASE.
- THERE SHALL BE NO DECORATIVE STONE, LANDSCAPING OR TREES PLANTED WITHIN THE COUNTY R.O.W.
- ALL TRAFFIC SIGNS PARKING THE COUNTY HIGHWAY SHALL BE INSTALLED BY THE DUPAGE COUNTY.
- THERMOPLASTIC PAVEMENT MARKING IS REQUIRED WITHIN LIMITS OF THE ACCESS DRIVEWAY AND IN THE 63RD ST. COUNTY RIGHT OF WAY.
- ALL EXISTING SIDEWALK DAMAGED IN THE 63RD ST. COUNTY RIGHT OF WAY SHALL BE REPLACED IN LIKE KIND.

OVERALL PARKING DATA			
	EXISTING	PROPOSED	
SHOPPING CENTER	774 SPACES	SHOPPING CENTER	651 SPACES
ROUNDHEAD'S PIZZA	21 SPACES	WALGREENS	43 SPACES
TOTAL	795 SPACES	TOTAL	694 SPACES

SITE DATA	
TOTAL PROPERTY AREA	1.60 ACRES
LOT 4 (OUTLOT) AREA	0.52 ACRES
LOT 3 (WALGREENS) AREA	1.08 ACRES
PARKING PROVIDED	43 SPACES
HANDICAP PROVIDED	2 SPACES

- SIGN LEGEND**
- ① R1-1 STOP SIGN
 - ② R7-8 HANDICAP PARKING SIGN
 - ③ FIRE LANE - NO PARKING SIGN
 - ④ R5-1 DO NOT ENTER
 - ⑤ R6-25 ONE WAY
 - ⑥ R3-5R RIGHT TURN ONLY

PAVEMENT LEGEND

STANDARD DUTY PAVEMENT

- 1 1/2" BITUMINOUS SURFACE COURSE, HOT-MIX ASPHALT, MIX D, N50
- 2 1/4" BITUMINOUS BINDER COURSE, HOT-MIX ASPHALT, IL-19, N50
- 8" AGGREGATE BASE COURSE, TYPE B

HEAVY DUTY PAVEMENT

- 2" BITUMINOUS SURFACE COURSE, HOT-MIX ASPHALT, MIX D, N50
- 3" BITUMINOUS BINDER COURSE, HOT-MIX ASPHALT, IL-19, N50
- 10" AGGREGATE BASE COURSE, TYPE B

WALGREENS CONCRETE PAVEMENT

- 6 1/2" PORTLAND CEMENT CONCRETE PAVEMENT W/ 6 X 6 W1.4 WWF
- 4" COMPACTED AGGREGATE BASE, TYPE B

DUPAGE COUNTY D.O.T. DRIVEWAY APRON CONCRETE

- 1 1/2" BITUMINOUS CONCRETE SURFACE COURSE
- 1 1/2" BITUMINOUS CONCRETE BINDER COURSE
- 6" BITUMINOUS CONCRETE BASE MATERIAL (B.A.M.)
- 8" CA-6 (GRADE 8) COMPACTED GRAVEL SUB-BASE

THICKENED CONCRETE CURB & GUTTER

- 8" PORTLAND CEMENT CONCRETE PAVEMENT W/ 6 X 6 W1.4 WWF
- 8" CA-6 (GRADE 8) COMPACTED GRAVEL SUB-BASE

CONCRETE SIDEWALK

- 5" PORTLAND CEMENT CONCRETE PAVEMENT W/ 6 X 6 W1.4 WWF
- 4" COMPACTED AGGREGATE BASE, TYPE B

B6.12 CONCRETE CURB & GUTTER

REVERSE PIT/B6.12 CONCRETE CURB & GUTTER

- PAVEMENT MARKING LEGEND**
- ① 24" WHITE STOP BAR
 - ② 4" YELLOW LINE
 - ③ 6" SOLID WHITE
 - ④ LETTERS AND SYMBOLS PAVEMENT MARKINGS
 - ⑤ 6" WHITE - 2" DASH 6" SKIP
 - ⑥ 4" YELLOW DIAGONAL AT 45° SPACED 2' O.C. W/ 4" YELLOW BORDER
 - ⑦ 12" YELLOW DIAGONAL AT 45° (3 SPACED EVENLY) W/ DOUBLE 4" YELLOW (11" O.C.) BORDER

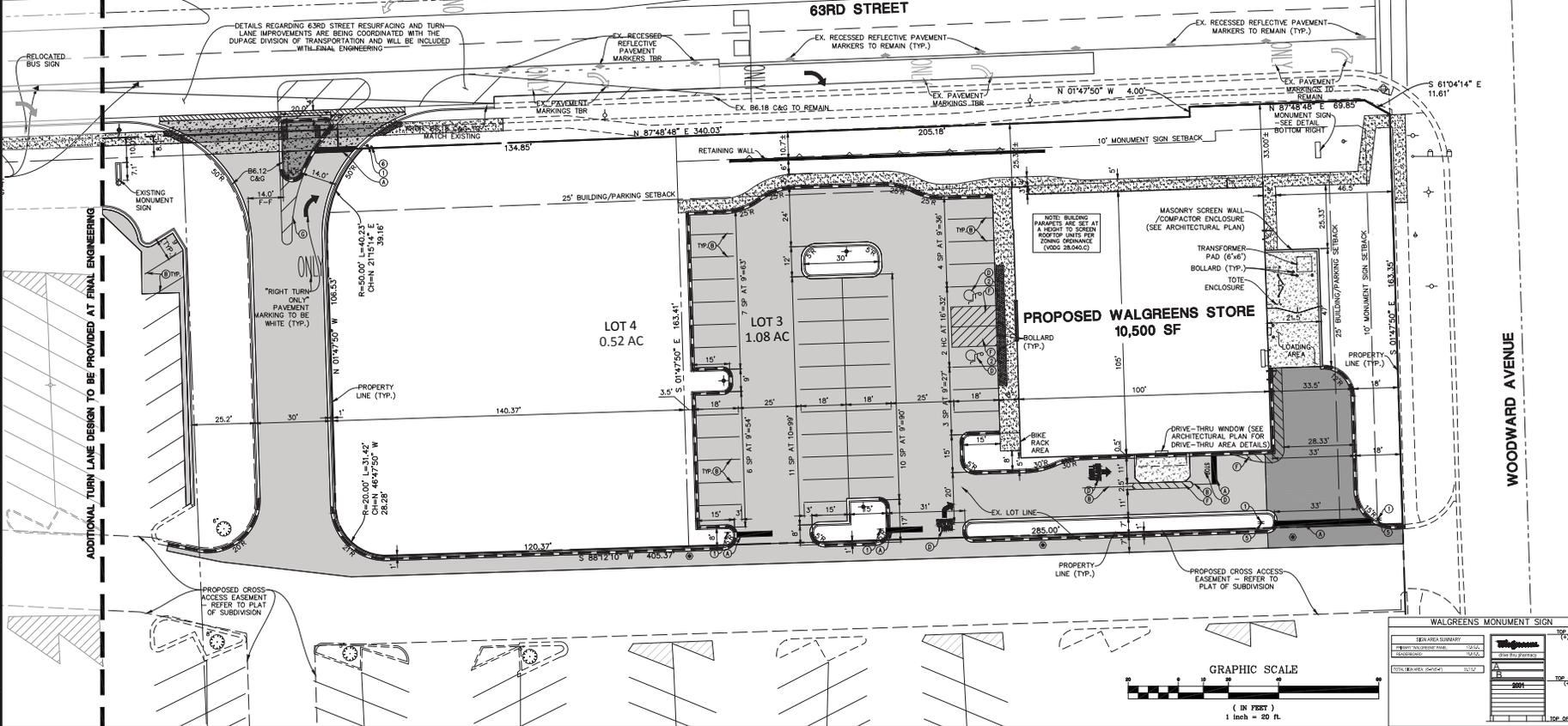
CONTRACTOR SHALL REFER TO ARCHITECTURAL PLANS FOR EXACT LOCATION OF SIDEWALKS, SIDEWALK SCORING, BENCHES, BIKE RACKS, FLAG POLES, ETC. DIMENSIONS OF VESTIBLES, RAMPS AND TRUCK DOCKS, PRECISE BUILDING DIMENSIONS AND EXACT UTILITY ENTRANCE LOCATIONS.

CONTROL POINT FOR LAYOUT OF ALL PROPOSED IMPROVEMENTS IS THE SOUTHEAST PROPERTY CORNER. BUILDING AND PARKING TO BE PARALLEL AND PERPENDICULAR TO EAST PROPERTY LINE UNLESS OTHERWISE NOTED.

- BENCHMARK:**
- SOURCE BENCHMARK 1:**
 DUPAGE COUNTY 2006 GEODETIC SURVEY MONUMENT 0214, PID DK3151, DESCRIBED AS BRASS DISK LOCATED IN THE CONCRETE BASE OF A LIGHT POLE LOCATED 21' SOUTH OF THE CENTERLINE OF 71ST STREET AND 70' WEST OF THE CENTERLINE OF BINDER ROAD.
 ELEVATION=774.53 NAVD 88
- SOURCE BENCHMARK 2:**
 DUPAGE COUNTY 2006 GEODETIC SURVEY MONUMENT, PID MPT251, DESCRIBED AS TOP OF A STEEL ROD LOCATED IN PVC SLEEVE WITH BERTINSEN UN LOCATED 145' SOUTH OF THE CENTERLINE OF 63RD STREET AND 42' WEST OF CENTERLINE OF DUNHAM ROAD.
 ELEVATION=745.59 NAVD 88
- SITE BENCHMARK 1:**
 TAG BOLT OF FIRST FIRE HYDRANT WEST OF WOODWARD AVENUE ON THE SOUTH SIDE OF 63RD STREET.
 ELEVATION=738.96 NAVD 88
- SITE BENCHMARK 2:**
 TAG BOLT OF SECOND HYDRANT WEST OF WOODWARD AVENUE ON THE SOUTH SIDE OF 63RD STREET.
 ELEVATION=744.51 NAVD 88

Project Name: 63RD STREET AND WOODWARD AVENUE
 Address: 2001 63rd Street
 PIN: 08-24-202-009
 Zoning District: R-2
 Existing Use: Commercial Restaurant (Roundhead's Pub)
 Proposed Use: Commercial Retail (Walgreens)
 Petition Type: Planned Unit Development / Special Use Permits
 Deviations: Proposed Drive - Thru (Authorized Special Use in Zone B-2)

Requirement	Factor	Required	Proposed/Existing	Meets Req?	Difference
Lot Frontage	Minimum	-	125.00' (Lot 4) - 454.00' (Lot 3)	-	-
Lot Area	Minimum	-	22,615.50 (Lot 4) - 47,138.50 (Lot 3)	-	-
Lot Width	Minimum	-	140.37' (Lot 4) - 285.00' (Lot 3)	-	-
Street Yard	Minimum	25'	25.32'	Yes	+0.32'
Rear Yard	Minimum	-	-	-	-
Side Yard	Minimum	-	-	Yes	-
Height	Maximum	35'	29' 4"	Yes	-5' 8"
Open Space	Minimum	10%	24% (Lot 3)	Yes	+14%
FAR	Maximum	0.75	0.22 (Lot 3)	Yes	-0.53
Parking	Minimum	3.5 Spaces per 1,000 sq ft (37)	4.5 Spaces per 1,000 sq ft (43)	Yes	+6 spaces
Donations*	Minimum	-	-	-	-



WALGREENS MONUMENT SIGN

SIGN AREA SURVEY	TOP OF SIGN
GROUND SURVEY	TOP OF FOUNDATION
DATE: 12-20-17	SCALE: 1"=20'
DRAWN BY: HCM	SHEET: C-300
DATE: 12-20-17	PREPARED BY: FREDG

Manhard CONSULTING LTD.

PROPOSED WALGREENS - PRELIMINARY
 VILLAGE OF DOWNERS GROVE, ILLINOIS
 SITE DIMENSIONAL AND PAVING PLAN

PRELIMINARY PLAN - NOT FOR CONSTRUCTION

DATE: _____
 DRAWN BY: _____
 CHECKED BY: _____
 DATE: 12-20-17
 SCALE: 1"=20'
 SHEET: C-300
 PREPARED BY: FREDG

GRADING NOTES:

- RETAINING WALL DESIGN TO BE PROVIDED BY OTHERS.
- PAVEMENT SLOPES THROUGH HANDICAP ACCESSIBLE PARKING AREAS SHALL BE 2.00% MAXIMUM IN ANY DIRECTION.
- ALL HANDICAP RAMPS SHALL BE CONSTRUCTED WITH A MAXIMUM CROSS SLOPE OF 2.00% OR LESS.
- MEET EXISTING GRADE AT PROPERTY LIMITS UNLESS NOTED OTHERWISE.
- CONTRACTOR SHALL REFER TO THE SOIL EROSION AND SEDIMENT CONTROL PLAN AND DETAILS FOR CONSTRUCTION SCHEDULING AND EROSION CONTROL MEASURES TO BE INSTALLED PRIOR TO BEGINNING GRADING OPERATIONS.
- THE CONTRACTOR SHALL CONTACT JULLIE, (1-800-892-0123) PRIOR TO ANY WORK TO LOCATE UTILITIES AND SHALL CONTACT THE OWNER SHOULD UTILITIES APPEAR TO BE IN CONFLICT WITH THE PROPOSED IMPROVEMENT.
- THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES, AND WHERE POSSIBLE MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANIES AT LEAST 72 HOURS BEFORE ANY EXCAVATION TO RELOCATE EXACT FIELD LOCATION OF UTILITIES. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE PLANS.
- IF ANY EXISTING STRUCTURES TO REMAIN ARE DAMAGED DURING CONSTRUCTION IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO REPAIR AND/OR REPLACE THE EXISTING STRUCTURE AS NECESSARY TO RETURN IT TO EXISTING CONDITION OR BETTER.
- ALL UNSURFACED AREAS DISTURBED BY GRADING OPERATION SHALL RECEIVE 6 INCHES OF TOPSOIL. CONTRACTOR SHALL APPLY STABILIZATION FABRIC TO ALL SLOPES 3% OR STEEPER. CONTRACTOR SHALL STABILIZE DISTURBED AREAS IN ACCORDANCE WITH GOVERNING SPECIFICATIONS UNTIL A HEALTHY STAND OF VEGETATION IS OBTAINED.
- EXISTING TOPOGRAPHY SHOWN REPRESENTS SITE CONDITIONS AS PREPARED BY MANHARD CONSULTING LTD ON MARCH 15, 2016. CONTRACTOR SHALL FIELD CHECK EXISTING ELEVATIONS AND CONDITIONS PRIOR TO CONSTRUCTION AND NOTIFY ARCHITECT AND ENGINEER OF ANY DISCREPANCIES PRIOR TO STARTING CONSTRUCTION. IF THE CONTRACTOR DOES NOT ACCEPT EXISTING TOPOGRAPHY AS SHOWN ON THE PLANS, WITHOUT EXCEPTION, THEN THE CONTRACTOR SHALL SUPPLY, AT THEIR EXPENSE, A TOPOGRAPHIC SURVEY BY A REGISTERED LAND SURVEYOR TO THE OWNER FOR REVIEW.
- TRANSITIONS FROM DEPRESSED CURB TO FULL HEIGHT CURB SHALL BE TAPERED AT 2H:1V UNLESS OTHERWISE NOTED.

VILLAGE OF DOWNERS GROVE:

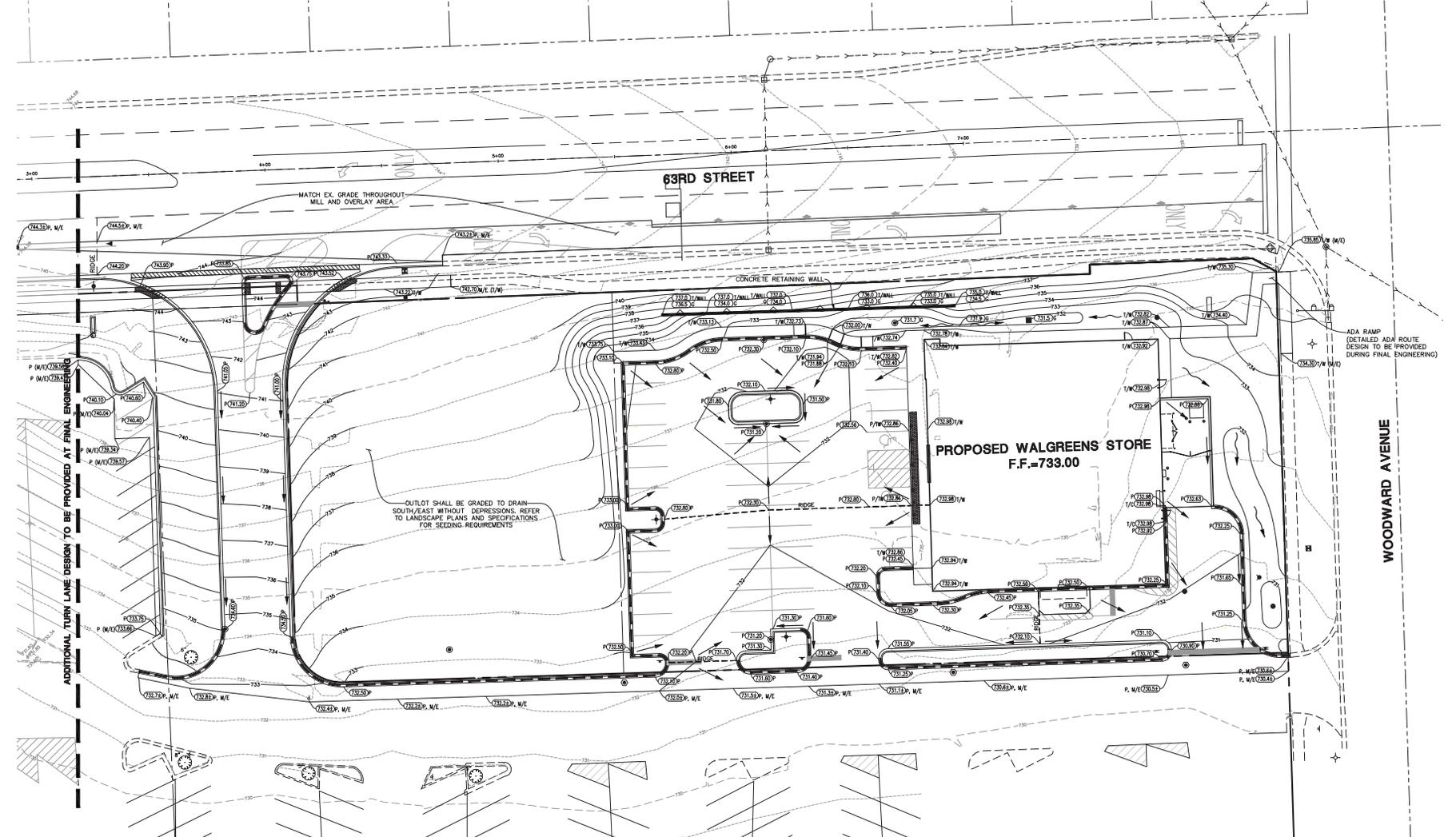
GENERAL NOTES:

- A FINAL GRADING SURVEY IS REQUIRED AT THE COMPLETION OF THE PROJECT, INCLUDING AN ELECTRONIC COPY. IT SHALL INCLUDE, BUT IS NOT LIMITED TO, THE FOLLOWING ITEMS:
 - SUMP PUMP DISCHARGE LOCATION, DISCHARGE PATH, AND THE LOCATION, SIZE, AND MATERIAL OF ANY ASSOCIATED PIPING. (SUMP PUMP DISCHARGE SHALL BE AT LEAST 20 FEET FROM THE DOWNSTREAM PROPERTY LINE)
 - DOWNSPOUT LOCATION, DISCHARGE PATH, AND THE LOCATION, SIZE, AND MATERIAL OF ANY ASSOCIATED PIPING. (DOWNSPOUT DISCHARGE SHALL BE AT LEAST 20 FEET FROM THE DOWNSTREAM PROPERTY LINE)
 - TOP OF FOUNDATION ELEVATIONS OF ALL NEW STRUCTURES
 - SPOT GRADES ADJACENT TO THE FOUNDATIONS OF ALL NEW STRUCTURES
 - ALL NEW IMPERVIOUS AREAS INCLUDING THOSE MADE OF CONCRETE, ASPHALT, AND BRICK
 - STOOPS OUTSIDE OF DOORWAYS
 - WINDOW WELL LOCATIONS, ELEVATIONS, AND ADJACENT GRADE
 - UPDATED CALCULATIONS OF THE AS-BUILT IMPERVIOUS AREAS, TABULATED TO SHOW THE NET INCREASE IN IMPERVIOUS AREA. (ANY INCREASE IN IMPERVIOUS AREA FROM THE PROPOSED WILL RESULT IN AN ADDITIONAL FEE)

GRADING PLAN LEGEND

- PROPOSED 1 FOOT CONTOURS
- PROPOSED SPOT ELEVATION
- PROPOSED FINISHED FLOOR ELEVATION
- PROPOSED GRADE AT FOUNDATION
- PROPOSED PAVEMENT ELEVATION
- PROPOSED TOP OF CURB
- PROPOSED TOP OF WALK
- PROPOSED TOP OF WALL
- PROPOSED GROUND GRADE OR GROUND AT BASE OF RETAINING WALL
- PROPOSED SLOPE OR SWALE
- PROPOSED DIRECTION OF FLOW
- OVERFLOW RELIEF SWALE
- PROPOSED RIDGE LINE
- PROPOSED DEPTH OF PONDING
- RETAINING WALL
- PROPOSED SWALE LOW POINT
- PROPOSED SWALE SUMMIT

GRAPHIC SCALE



ADDITIONAL TURN LANE DESIGN TO BE PROVIDED AT FINAL ENGINEERING

OUTLOT SHALL BE GRADED TO DRAIN SOUTH/EAST WITHOUT DEPRESSIONS. REFER TO LANDSCAPE PLANS AND SPECIFICATIONS FOR SEEDING REQUIREMENTS

ADA RAMP (DETAILED ADA ROUTE DESIGN TO BE PROVIDED DURING FINAL ENGINEERING)

DATE: _____

REVISIONS:

NO.	DATE	DESCRIPTION

DATE: 12-20-17

SCALE: 1"=20'

SHEET: C-400

PREP: FREDG

Manhard CONSULTING LTD

Civil, Mechanical, Electrical, Plumbing, Fire, and Energy Engineers and Architects

DATE: 12-20-17

SCALE: 1"=20'

SHEET: C-400

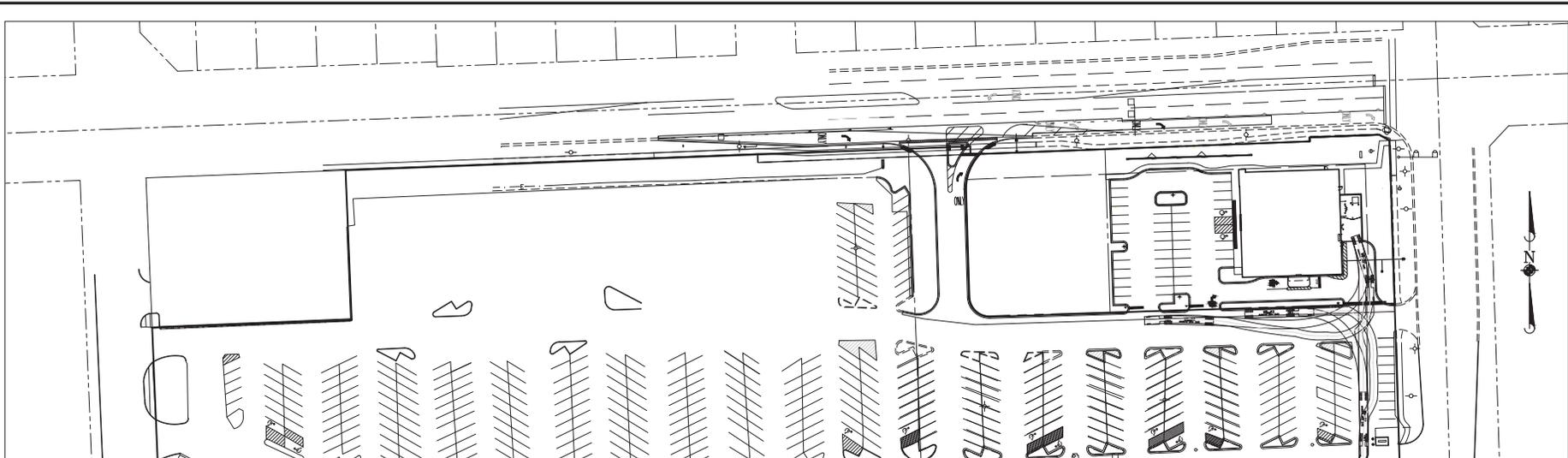
PREP: FREDG

PROPOSED WALGREENS - PRELIMINARY

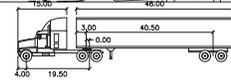
VILLAGE OF DOWNERS GROVE, ILLINOIS

GRADING PLAN

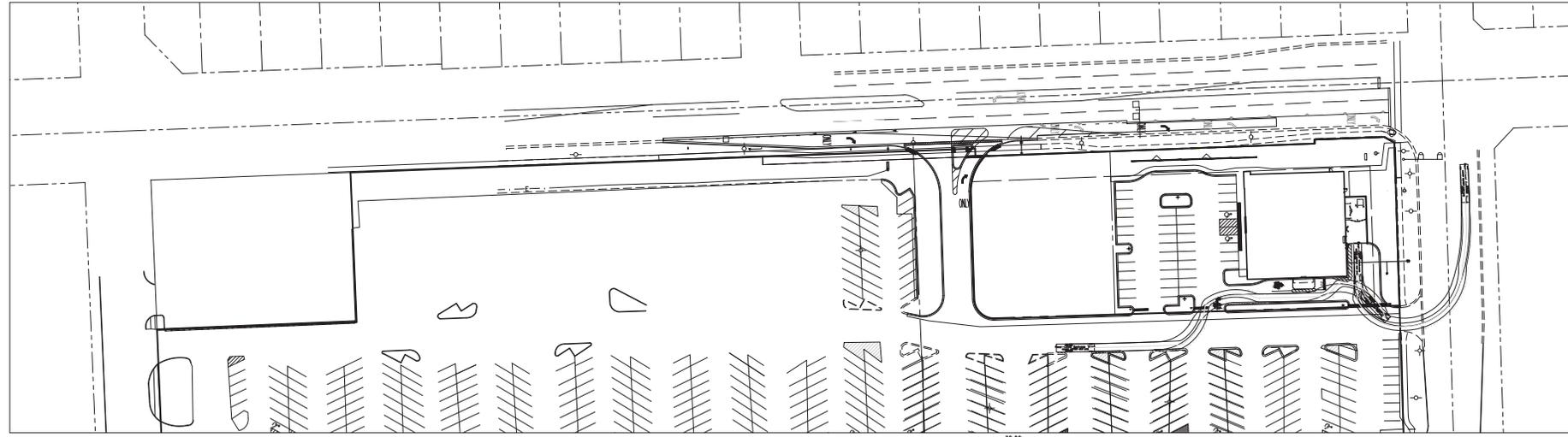
PRELIMINARY PLAN - NOT FOR CONSTRUCTION



WB-62 TRUCK



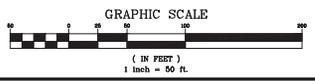
WB-62		feet	
Tractor Width	: 8.00	Lock to Lock Time	: 6.0
Tractor Wheel	: 8.50	Steering Angle	: 28.4
Tractor Track	: 8.00	Articulating Angle	: 70.0
Trailer Track	: 8.50		



REAR-LOAD GARBAGE TRUCK



Rear-Load Garbage Truck		feet	
Width	: 3.58	Track to Lock Time	: 6.0
Track	: 8.00	Steering Angle	: 27.4
Lock to Lock Time	: 6.0		
Steering Angle	: 27.4		



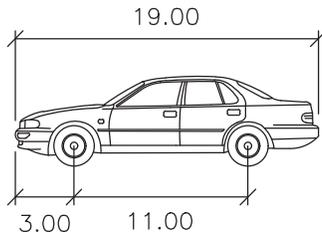
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 101-118-118 REVISED PER VALUAGE REVIEW

WALGREENS
 VILLAGE OF DOWNERS GROVE
 AUTOTURN EXHIBIT

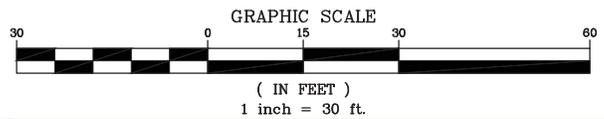
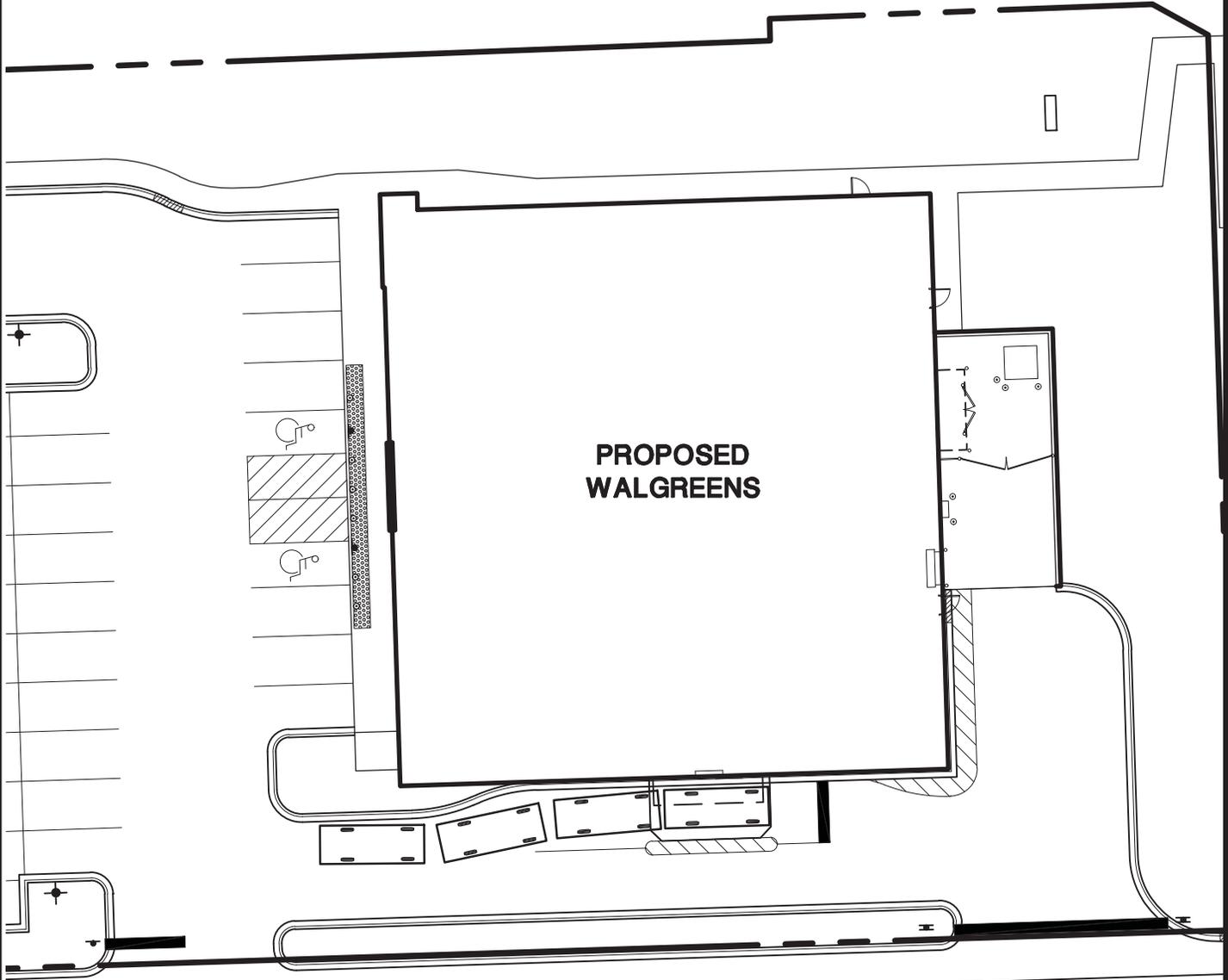
PROJ. NO.: SMS
 PROJ. ASSOC.: HCM
 DRAWN BY: HCM
 DATE: 12-20-17
 SCALE: 1"=50'

SHEET
1 OF **2**
 FREDG

Date: 12/20/17 10:45 AM
 Drawn By: HCM
 Project: Walgreens Village of Downers Grove Autoturn Exhibit
 Scale: 1"=50'
 Sheet: 1 of 2
 Project: Walgreens Village of Downers Grove Autoturn Exhibit



P feet
 Width : 7.00
 Track : 6.00
 Lock to Lock Time : 6.0
 Steering Angle : 31.6



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700 Springer Drive, Lombard, IL 60148 ph: 630.691.6500 fx: 630.691.6565 manhard.com
 Civil Engineers • Surveyors • Water Resource Engineers • Water & Wastewater Engineers
 Construction Managers • Environmental Scientists • Landscape Architects • Planners

WALGREENS	
VILLAGE OF DOWNERS GROVE	
DRIVE THRU STACKING EXHIBIT	
PROJ. MGR.: <u>SMS</u>	SHEET
DRAWN BY: <u>HCM</u>	EXHIBIT 1
DATE: <u>01/29/17</u>	FREDG
SCALE: <u>1"=30'</u>	

Dwg Name: P:\Fredg\dwg\Eng\Final Drawings\Exhibits Eng\DriveThruStacking Exhibit New.dwg Updated By: hmeyer 14:46



MEMORANDUM TO: Mitchell P Kahn
Frontline Real Estate Partners, LLC

FROM: Javier Millan
Senior Consultant

Luay Aboona, PE
Principal

DATE: December 15, 2017

SUBJECT: Trip Generation Comparison
Revised Walgreens Development Plan
Downers Grove, Illinois

This memorandum provides a comparison of the estimated traffic to be generated by the (1) approved plan and (2) current proposed plan for the Walgreens Drive-Through Pharmacy store to be located in the southwest corner of the intersection of 63rd Street with Woodward Avenue within the Meadowbrook shopping center in Downers Grove, Illinois. The approved development plan included an approximate 14,500 square-foot Walgreens pharmacy with drive-through facility. As currently proposed, the development plan calls for a smaller Walgreens of approximately 10,500 square feet to be located on the east end of the parcel. The west end of the parcel will be developed by others at a later time. Access to the Walgreens pharmacy will continue to be provided via the existing access drives serving the Meadowbrook shopping center.

The number of peak hour vehicle trips estimated to be generated by the approved development plan and the current proposed development plan were based on trip data for land use code 881 (Pharmacy/Drugstore w/Drive-Through) published by the Institute of Transportation Engineers (ITE) in its *Trip Generation Manual*, 9th Edition. **Table 1** shows the traffic estimated to be generated by the approved development plan and the current proposed development plan. With the reduction in size, it can be seen that the current proposed development plan is estimated to generate less traffic than the approved development plan. As such, the findings and conclusions of the original traffic impact evaluation dated February 2, 2017 remain.

Table 2
EXISTING AND ESTIMATED TRAFFIC VOLUMES – PROPOSED WALGREENS

Land-Use Code	Type/Size	Weekday Morning Peak Hour			Weekday Evening Peak Hour			Saturday Midday Peak Hour			Daily Two-Way Traffic
		In	Out	Total	In	Out	Total	In	Out	Total	Total
Approved Development Plan											
881	Pharmacy/Drugstore w/ Drive-Through (14,500 s.f.)	26	24	50	72	72	144	59	61	120	1,406
	<i>Pass-By Trip Reduction (50%):</i>	<u>-13</u>	<u>-12</u>	<u>-25</u>	<u>-36</u>	<u>-36</u>	<u>-72</u>	<u>-30</u>	<u>-30</u>	<u>-60</u>	<u>-702</u>
Total New Trips Generated:		13	12	25	36	36	72	29	31	60	704
Current Proposed Development Plan											
881	Pharmacy/Drugstore w/ Drive-Through (10,500 s.f.)	19	17	36	52	52	104	42	44	86	1,018
	<i>Pass-By Trip Reduction (50%):</i>	<u>-10</u>	<u>-8</u>	<u>-18</u>	<u>-26</u>	<u>-26</u>	<u>-52</u>	<u>-21</u>	<u>-22</u>	<u>-43</u>	<u>-509</u>
Total New Trips Generated:		9	9	18	26	26	52	21	22	43	509
Difference		-4	-3	-7	-10	-10	-20	-8	-9	-17	-195



MEMORANDUM TO: Mitchell P. Kahn
Frontline Real Estate Partners, LLC

FROM: Javier Milan
Senior Consultant

Luay R. Aboona, PE
Principal

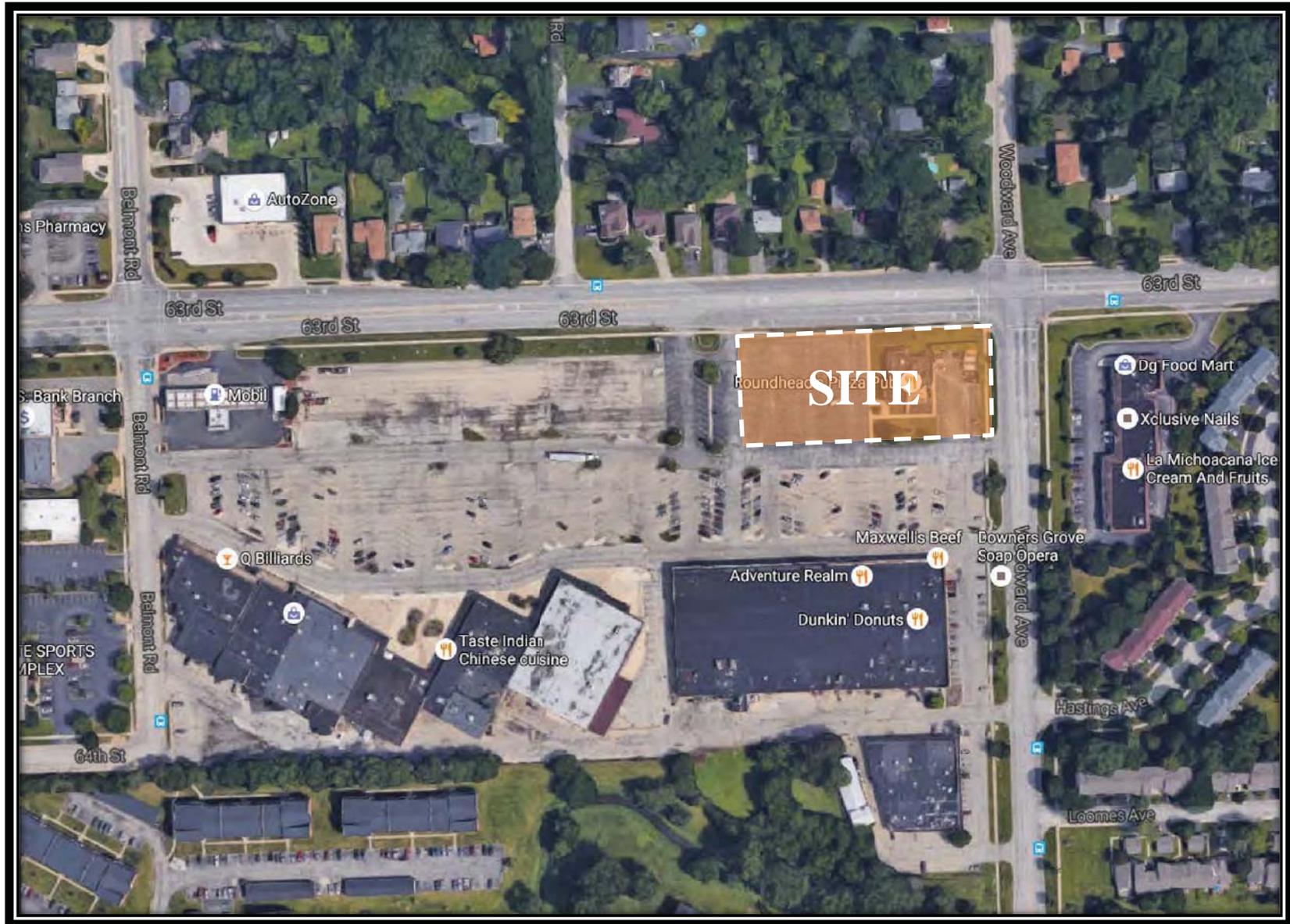
DATE: February 2, 2017

SUBJECT: Traffic Impact Evaluation
Proposed Walgreens Drive-Through Pharmacy Store
Downers Grove, Illinois

This memorandum summarizes the results of a traffic impact evaluation conducted by Kenig, Lindgren, O'Hara, Aboona, Inc. (KLOA, Inc.) for the proposed Walgreens Drive-Through Pharmacy Store in Downers Grove, Illinois. The plans call for relocating the existing Walgreens store in the northwest corner of the intersection of 63rd Street with Belmont Avenue to the southwest corner of the intersection of 63rd Street with Woodward Avenue. The proposed location, which is currently occupied by Roundhead's Pizza Pub, is located within the Meadowbrook shopping center. **Figure 1** shows an aerial view of the site area.

The purpose of this evaluation is to address concerns raised regarding existing traffic operations at the intersection of 63rd Street with Woodward Avenue, which include the following:

- Queueing and delays experienced by traffic on Woodward Avenue
- The impact of the additional traffic that the proposed Walgreens development will generate
- The projected increase in traffic on Woodward Avenue north of 63rd Street



Aerial View of Site Location

Figure 1

Existing Roadways

The existing roadways and traffic control characteristics of the adjacent roadways are described below.

63rd Street (DuPage County Route 38) is an east-west roadway with a five-lane cross section that in the vicinity of the site provides two lanes in each direction divided by a striped median. At its signalized intersection with Woodward Avenue, 63rd Street provides an exclusive left-turn lane, two exclusive through lanes, and an exclusive right-turn lane on the eastbound approach and an exclusive left-turn lane, an exclusive through lane, and a shared through/right-turn lane on the westbound approach. Standard style crosswalks are provided on all legs of the intersection along with pedestrian signals. 63rd Street is under the jurisdiction of the DuPage County Division of Transportation (DuDOT) and has a posted speed limit of 40 miles per hour (mph).

Woodward Avenue is a north-south roadway that provides one lane in each direction north of 63rd Street and two lanes in each direction south of 63rd Street. At its signalized intersection with 63rd Street, Woodward Avenue provides a shared left-turn/through lane and a shared through/right-turn lane on the southbound approach and an exclusive left-turn lane, a shared left-turn/through lane, and an exclusive right-turn lane on the northbound approach. Woodward Avenue is under the jurisdiction of Lisle Township and Downers Grove Township north of 63rd street and the Village of Downers Grove south of 63rd Street. Woodward Avenue has a posted speed limit of 25 mph north of 63rd Street increasing to 30 mph south of 63rd Street. Through traffic and trucks over eight tons are prohibited on Woodward Avenue via signage north of 63rd Street.

Existing Traffic Volumes

In order to determine current traffic conditions at the intersection of 63rd Street with Woodward Avenue, KLOA, Inc. conducted peak period traffic counts on Saturday, January 21, 2017 and on Tuesday, January 24, 2017 during the weekday morning (7:00 A.M. to 9:00 A.M.) and weekday evening (2:30 P.M. to 6:00 P.M.) peak periods and on Saturday January 21, 2017 during the midday (12:00 P.M. to 2:00 P.M.) peak period. The results of the traffic counts showed that the weekday morning peak hour of traffic occurs from 7:15 A.M. to 8:15 A.M., the weekday evening peak hour of traffic occurs from 4:00 P.M. to 5:00 P.M., and the Saturday midday peak hour of traffic occurs from 12:45 P.M. to 1:45 P.M. **Figure 2** illustrates the existing peak hour traffic volumes. Summaries of the traffic counts can be found in the Appendix.

In addition, the results of the traffic counts were compared with counts previously conducted by DuDOT in 2014 and were found to be generally consistent. It should also be noted that the traffic counts were conducted while the nearby Indian Trail Elementary School was in session and school-related traffic was included in the traffic counts. A review of the traffic counts showed that approximately 30 percent more traffic traveled through the intersection of 63rd Street with Woodward Avenue during the evening peak hour (5:00 P.M. to 6:00 P.M.) than during the afternoon peak hour (2:30 P.M. to 3:30 P.M.) when the school ends. As such, the higher evening traffic volumes were utilized in the evaluation.

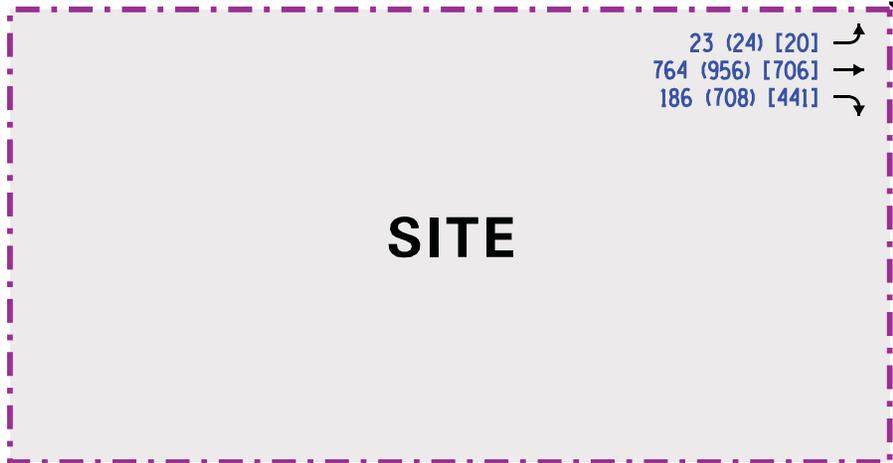


NOT TO SCALE

21 (22) [181]
37 (140) [551]
11 (16) [171]

17 (5) [6]
860 (828) [824]
106 (289) [189]

63RD
STREET



23 (24) [20]
764 (956) [706]
186 (708) [441]

328 (191) [234]
81 (49) [39]
812 (336) [411]

WOODWARD
AVENUE

LEGEND

-  - TRAFFIC SIGNAL
- 00 - AM PEAK HOUR (7:15-8:15 AM)
- (00) - PM PEAK HOUR (5:00-6:00 PM)
- [00] - SATURDAY MIDDAY PEAK HOUR (12:45-1:45 PM)

4

PROJECT:
Proposed
Walgreens Development
Downers Grove, Illinois

TITLE:
Existing Traffic Volumes

KLOA
Job No: 16-297

Figure: 2

Traffic Operations of the 63rd Street/Woodward Avenue Intersection

The intersection of 63rd Street with Woodward Avenue is under traffic signal control with split phasing for the northbound and southbound approaches. This intersection is part of a coordinated system on 63rd Street that extends from Main Street to the east to Leonard Avenue to the west. The intersection of 63rd Street with Woodward Avenue is fully actuated on all approaches and provides protected/permissive left-turn phases on the eastbound and westbound approaches and right-turn overlap phases on the eastbound and northbound approaches. A sign facing southbound on Woodward Avenue north of 63rd Street prohibits non-local traffic on Woodward Avenue. However, no such signs are provided on Woodward Avenue at its intersection with Maple Avenue or on 61st Street or 59th Street at their respective intersections with Belmont Avenue. As such, cut-through traffic is only prohibited from traveling northbound on Woodward Avenue, not southbound.

Capacity analyses were conducted at the intersection under existing conditions utilizing the existing signal timings and phasing. The results of the capacity analyses expressed in terms of Level of Service (LOS) and average delays are summarized in **Table 1**. As can be seen, the intersection overall operates at an acceptable LOS C during all three peak hours. However, the northbound and southbound approaches operate at LOS D/E which is primarily due to the limited green time allocated to these approaches, the split phase, and the high volume of traffic on northbound Woodward Avenue.

The results of the capacity analyses were also confirmed by the following observations that were made of existing conditions:

- Morning Peak Hour
 - Traffic queues on northbound Woodward Avenue were consistently observed to extend between Hastings Avenue and Loomes Avenue for approximately 30 to 60 seconds; however, the queues cleared the intersection most of the time with each green phase.
 - Traffic queues on southbound Woodward Avenue were observed to consist of a maximum of four to eight vehicles each cycle and cleared the intersection with each green phase.
- Evening Peak Hour
 - Traffic queues on southbound Woodward Avenue were observed to consistently extend past the shared left-turn/through lane storage length and taper with combined lane queues of approximately 10 to 16 vehicles per cycle.
 - A significant portion of traffic traveling on Woodward Avenue north of 63rd Street during the peak hours was observed to be cut-through traffic traveling to/from Belmont Avenue to the west and Maple Avenue from the north.

- Based on the previous observation, the sign prohibiting non-local traffic on northbound Woodward Avenue north of 63rd Street is not being adhered to.
- Additional signage prohibiting cut-through traffic needs to be posted at 59th Street, 61st Street, and Maple Avenue.

The following is a summary of the reasons for the long delays and queues experienced at this intersection:

- The north-south split phasing nature of the intersection is the primary cause for the intersection's poor level of service, extensive queueing, and significant delay.
 - The split phasing at this intersection is required because of the striping on the south leg to accommodate the high volume of northbound left-turn movements and the limitation of sufficient right-of-way to geometrically improve the intersection to accommodate the existing traffic volumes without split phasing.
 - The northbound and southbound phases are allocated a proportionate amount of green time based on their respective traffic volumes which results in a minimal amount of green time for the southbound phase (approximately 8, 15, and 13 seconds during the weekday morning, weekday evening, and Saturday midday peak hours, respectively), thus resulting in the delays on the approach.
- The cause for the high volume of northbound left-turn movements on Woodward Avenue and the high volume of eastbound right-turn movements on 63rd Street is the result of how the area roadways are configured.
 - As previously mentioned, Woodward Avenue north of 63rd Street is restricted to local traffic only and does not allow vehicles over eight tons.
 - Vehicles traveling on Woodward Avenue that desire to continue to travel northbound must turn left onto westbound 63rd Street and then turn right onto northbound Belmont Street approximately one-quarter mile to the west of Woodward Avenue.
 - These two north-south streets act as non-continuous arterial roadways. Instead of being continuous or directly connected, traffic must travel on 63rd Street to continue to travel either north or south via Woodward or Belmont Avenue.

Table 1

CAPACITY ANALYSIS RESULTS – 63RD STREET WITH WOODWARD AVENUE – SIGNALIZED

	Peak Hour	Eastbound			Westbound			Northbound			Southbound			Overall
		L	T	R	L	T	R	L	T	R	L	T	R	
Existing Conditions	Weekday Morning Peak Hour	C 22.8	C 23.0	A 6.3	C 21.6	C 22.7	C 23.0	E 60.1	E 59.5	D 37.3	E 62.1		E 61.8	C – 34.4
		B – 19.8			C – 22.7			D – 53.8			E – 61.9			
	Weekday Evening Peak Hour	B 13.8	B 12.9	B 17.0	B 14.7	A 7.4	A 7.5	E 60.7	E 64.6	D 44.5	E 69.3		E 67.0	C – 22.4
		B – 14.6			A – 9.3			E – 56.7			E – 68.2			
	Saturday Midday Peak Hour	B 14.2	B 13.4	B 10.7	B 13.1	B 11.1	B 11.2	D 47.7	D 47.3	D 37.8	D 51.7		D 51.6	C – 20.6
		B – 12.4			B – 11.5			D – 44.2			D – 51.7			

Development Traffic Generation

The traffic to be generated by the proposed development was estimated using trip data published by the Institute of Transportation Engineers (ITE) in its *Trip Generation Manual*, 9th Edition. The trip rates were applied for the weekday morning and evening peak hours and on a daily basis for a Pharmacy/Drugstore with Drive-Through Window (Land-Use Code 881). In addition, the traffic currently generated by the existing Walgreens was observed and the resulting trip generation was compared with the ITE estimates. **Table 2** shows the trip generation comparison, which indicates that the estimated trips are very similar. It is important to note that surveys conducted by ITE have shown that up to 50 percent of trips made to pharmacy/drugstores with drive-through are diverted from the existing traffic on the roadway system. Such diverted trips are referred to as pass-by traffic. As such, a 50 percent pass-by reduction was applied to the trip generation estimates of the proposed development.

It is also important to note that the proposed Walgreens is a relocation of the existing Walgreens store located approximately one block west of the site. As such, the majority of its traffic is already traversing the intersection of 63rd Street with Woodward Avenue and as a result is not expected to add a significant amount of new traffic to the intersection, with its current operations expected to remain largely unchanged. However, in order to provide a conservative analysis, the traffic that will be generated by the proposed Walgreens was assumed to all be new to the area roadways.

In order to project Year 2018 conditions, existing traffic volumes on 63rd Street and Woodward Avenue were increased by one percent based on projections provided by the Chicago Metropolitan Agency for Planning (CMAP). In addition, traffic to be generated by the proposed development was assigned to the roadways as determined from the traffic counts. The assignment of traffic was determined as follows:

- 40 percent traveling to and from the west on 63rd Street
- 30 percent traveling to and from the east on 63rd Street
- 30 percent traveling to and from the south on Woodward Avenue
- Five percent traveling to and from the north of Woodward Avenue

The Year 2018 projected conditions for the intersection of 63rd Street with Woodward Avenue were analyzed. **Table 3** summarizes the intersection's LOS and delay for Year 2018 projected conditions during the peak hours.

Table 2

EXISTING AND ESTIMATED TRAFFIC VOLUMES – PROPOSED WALGREENS

Land-Use Code	Type/Size	Weekday Morning Peak Hour			Weekday Evening Peak Hour			Saturday Midday Peak Hour			Daily Two-Way Traffic
		In	Out	Total	In	Out	Total	In	Out	Total	Total
	Existing Walgreens Traffic	12	7	19	65	77	142	58	59	117	N/A
881	Pharmacy/Drugstore w/ Drive-Through (14,500 s.f.)	26	24	50	72	72	144	59	61	120	1,406
	<i>Pass-By Trip Reduction (50%):</i>	<u>-13</u>	<u>-12</u>	<u>-25</u>	<u>-36</u>	<u>-36</u>	<u>-72</u>	<u>-30</u>	<u>-30</u>	<u>-60</u>	<u>702</u>
	Total New Trips Generated:	13	12	25	36	36	72	29	31	60	704

Table 3

CAPACITY ANALYSIS RESULTS – 63RD STREET WITH WOODWARD AVENUE – SIGNALIZED

	Peak Hour	Eastbound			Westbound			Northbound			Southbound			Overall
		L	T	R	L	T	R	L	T	R	L	T	R	
Projected Conditions	Weekday Morning Peak Hour	C 23.1	C 23.4	A 6.3	C 21.9	C 23.1	C 23.4	E 60.7	E 60.4	D 37.2	E 62.1		E 61.8	C – 34.8
		C – 20.2			C – 23.1			D – 54.3			E – 62.0			
	Weekday Evening Peak Hour	B 14.1	B 13.4	B 17.4	B 15.4	A 7.7	A 7.8	E 60.4	E 65.0	D 44.1	E 69.2		E 67.0	C – 22.7
		B – 15.1			A – 9.7			E – 56.6			E – 68.2			
	Saturday Midday Peak Hour	B 14.4	B 13.6	B 10.8	B 13.3	B 11.3	B 11.4	D 47.8	D 47.5	D 37.6	D 51.8		D 51.6	C – 20.8
		B – 12.6			B – 11.7			D – 44.2			D – 51.7			
LOS – Level of Service Delay is measured in seconds.														

As can be seen, the intersection is expected to continue to operate at an overall LOS C during the weekday morning, weekday evening, and Saturday midday peak hours with an increase in overall delay during each of the peak hours of one second or less. Similarly, the increase in delay of the southbound approach as a result of the increase in traffic will be minimal (less than one second). As such, the proposed development will have a minimal impact on the operation of the intersection.

Potential Intersection and Roadway Improvements

While the proposed relocation of Walgreens will have a negligible impact on the intersection, the following improvements to the intersection and/or roadways could be considered:

- In order to reduce cut through traffic and reduce queues on southbound Woodward Avenue, signs prohibiting non-local traffic should be placed on Woodward Avenue just south of Maple Avenue and on 61st Street and 59th Street just east of Belmont Avenue.
- In order to reduce delays on Woodward Avenue, additional green time could be allocated to the northbound and/or southbound movements.
 - Preliminary analysis showed that providing an additional five seconds of green time for the southbound approach during the evening peak hour would reduce delay for all southbound movements by approximately five to six seconds.
 - However, this will increase the overall intersection delay by approximately seven seconds during the evening peak hour.
 - It is important to note that reducing delay for the southbound approach may encourage additional cut-through traffic on Woodward Avenue.
 - Any change to signal timing will require DuDOT review and may not be approved due to its impact on 63rd Street traffic and the interconnect system.

Conclusion

Based on the preceding evaluation, the following conclusions are made:

- The proposed Walgreens is a relocation of the existing store located at the northwest corner of the intersection of 63rd Street with Belmont Avenue.
- The signalized intersection of 63rd Street with Woodward Avenue currently operates as a split phase intersection causing queues and delays on both the northbound and southbound approaches.
- Despite these delays, queues were generally observed to clear the intersection with each green phase.
- Cut-through traffic is utilizing Woodward Avenue to travel to/from Belmont Avenue to the west and Maple Avenue from the north.
- The proposed Walgreens will not add a significant amount of new traffic to 63rd Street or Woodward Avenue and, as such, will have a minimal impact on the operations of the intersection of 63rd Street with Woodward Avenue.
- A minimal amount of traffic generated by the proposed development will travel to/from the north on Woodward Avenue.
- Signs prohibiting cut-through traffic should be placed on Woodward Avenue just south of Maple Avenue and on 61st Street and 59th Street just east of Belmont Avenue to reduce the traffic volumes of Woodward Avenue north of 63rd Street.
- Traffic delays on the southbound approach of Woodward Avenue may be reduced by allocating additional green time which may cause increases in overall intersection delays. Such modifications would be subject to DuDOT review.

Appendix

Traffic Count Summary Sheets
Level of Service Criteria
Capacity Analysis Summary Sheets

Traffic Count Summary Sheets



Kenig Lindgren O'Hara Aboona, Inc.
9575 W. Higgins Rd., Suite 400
Rosemont, Illinois, United States 60018
(847)518-9990

Count Name: 63rd Street with Woodward
Avenue
Site Code:
Start Date: 01/21/2017
Page No: 1

Turning Movement Data

Start Time	63rd Street Eastbound					63rd Street Westbound					Woodward Avenue Northbound					Woodward Avenue Southbound									
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total
12:00 PM	0	1	161	120	0	282	0	50	175	2	0	227	0	89	9	65	0	163	0	1	22	3	1	26	698
12:15 PM	0	6	169	111	0	286	0	56	192	2	0	250	0	95	13	46	0	154	0	1	20	5	0	26	716
12:30 PM	0	5	197	111	0	313	0	61	170	4	0	235	0	100	11	60	2	171	0	2	18	4	0	24	743
12:45 PM	0	3	159	94	0	256	0	55	205	1	0	261	0	116	12	60	0	188	0	5	20	2	0	27	732
Hourly Total	0	15	686	436	0	1137	0	222	742	9	0	973	0	400	45	231	2	676	0	9	80	14	1	103	2889
1:00 PM	0	8	196	117	0	321	0	42	195	1	0	238	0	85	9	56	0	150	0	5	6	9	0	20	729
1:15 PM	0	2	178	104	0	284	0	46	202	2	0	250	0	102	10	51	1	163	0	4	16	1	0	21	718
1:30 PM	0	7	175	126	0	308	0	46	222	2	0	270	0	108	8	67	1	183	0	4	13	5	0	22	783
1:45 PM	0	1	191	107	1	299	0	47	150	2	0	199	0	98	7	65	0	170	0	3	6	4	0	13	681
Hourly Total	0	18	740	454	1	1212	0	181	769	7	0	957	0	383	34	239	2	666	0	16	41	19	0	76	2911
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
7:00 AM	0	1	179	37	0	217	0	16	212	2	0	230	0	194	9	50	0	253	0	0	7	1	0	8	708
7:15 AM	0	3	191	40	0	234	0	15	193	0	0	208	0	218	17	72	0	307	0	3	5	2	0	10	759
7:30 AM	0	7	194	55	0	256	0	26	232	2	0	260	0	213	26	85	0	324	0	3	14	4	0	21	861
7:45 AM	0	9	204	43	0	256	0	43	235	9	0	287	0	212	29	97	0	338	0	9	2	2	0	13	894
Hourly Total	0	20	768	175	0	963	0	100	872	13	0	985	0	837	81	304	0	1222	0	15	28	9	0	52	3222
8:00 AM	0	4	175	48	0	227	0	22	200	6	0	228	0	169	9	74	0	252	0	6	16	3	1	25	732
8:15 AM	0	9	149	51	0	209	0	40	212	1	0	253	0	141	14	53	0	208	0	5	10	4	1	19	689
8:30 AM	0	2	167	41	0	210	0	44	209	3	1	256	0	111	14	46	1	171	0	2	10	4	0	16	653
8:45 AM	0	3	156	54	0	213	0	24	180	0	0	204	0	112	4	35	0	151	0	1	11	2	0	14	582
Hourly Total	0	18	647	194	0	859	0	130	801	10	1	941	0	533	41	208	1	782	0	14	47	13	2	74	2656
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2:30 PM	0	5	149	104	0	258	0	51	167	2	0	220	0	71	9	50	0	130	0	1	11	2	0	14	622
2:45 PM	0	4	164	103	0	271	0	52	138	2	0	192	0	79	21	64	0	164	0	2	11	4	0	17	644
Hourly Total	0	9	313	207	0	529	0	103	305	4	0	412	0	150	30	114	0	294	0	3	22	6	0	31	1266
3:00 PM	0	5	145	112	0	262	0	65	139	7	0	211	0	68	11	40	0	119	0	4	20	4	2	28	620
3:15 PM	0	3	198	109	0	310	0	47	146	5	0	198	0	77	10	43	0	130	0	5	19	0	0	24	662
3:30 PM	0	4	185	129	0	318	0	67	194	4	1	265	0	87	8	68	0	163	0	2	20	1	1	23	769
3:45 PM	0	3	213	144	0	360	0	69	226	1	1	296	0	91	7	49	0	147	0	2	30	4	0	36	839
Hourly Total	0	15	741	494	0	1250	0	248	705	17	2	970	0	323	36	200	0	559	0	13	89	9	3	111	2890
4:00 PM	0	4	223	157	1	384	0	49	239	0	0	288	0	92	5	45	0	142	0	3	23	3	2	29	843
4:15 PM	0	3	234	157	0	394	0	69	157	1	0	227	0	102	6	43	0	151	0	0	26	3	0	29	801
4:30 PM	0	2	242	166	0	410	0	75	218	1	0	294	0	78	11	47	1	136	0	4	36	3	0	43	883
4:45 PM	0	2	220	146	0	368	0	63	211	1	0	275	0	90	10	43	0	143	0	0	16	6	0	22	808
Hourly Total	0	11	919	626	1	1556	0	256	825	3	0	1084	0	362	32	178	1	572	0	7	101	15	2	123	3335
5:00 PM	0	8	228	156	0	394	0	66	226	2	0	294	0	69	12	49	0	130	0	9	29	3	0	41	859
5:15 PM	0	6	238	178	0	422	0	95	197	1	0	293	0	96	13	50	0	159	0	5	42	0	0	47	921

5:30 PM	0	2	251	202	0	455	0	66	202	0	0	268	0	75	11	41	0	127	0	5	39	5	0	49	899
5:45 PM	0	8	239	168	0	415	0	62	203	2	0	267	0	96	13	51	0	160	0	3	30	8	0	41	883
Hourly Total	0	24	956	706	0	1686	0	289	828	5	0	1122	0	336	49	191	0	576	0	22	140	16	0	178	3562
Grand Total	0	130	5770	3292	2	9192	0	1529	5847	68	3	7444	0	3334	348	1665	6	5347	0	99	548	101	8	748	22731
Approach %	0.0	1.4	62.8	35.8	-	-	0.0	20.5	78.5	0.9	-	-	0.0	62.4	6.5	31.1	-	-	0.0	13.2	73.3	13.5	-	-	-
Total %	0.0	0.6	25.4	14.5	-	40.4	0.0	6.7	25.7	0.3	-	32.7	0.0	14.7	1.5	7.3	-	23.5	0.0	0.4	2.4	0.4	-	3.3	-
Lights	0	129	5601	3255	-	8985	0	1494	5682	64	-	7240	0	3292	339	1625	-	5256	0	93	537	97	-	727	22208
% Lights	-	99.2	97.1	98.9	-	97.7	-	97.7	97.2	94.1	-	97.3	-	98.7	97.4	97.6	-	98.3	-	93.9	98.0	96.0	-	97.2	97.7
Buses	0	0	112	27	-	139	0	18	83	2	-	103	0	22	6	35	-	63	0	4	10	2	-	16	321
% Buses	-	0.0	1.9	0.8	-	1.5	-	1.2	1.4	2.9	-	1.4	-	0.7	1.7	2.1	-	1.2	-	4.0	1.8	2.0	-	2.1	1.4
Single-Unit Trucks	0	1	50	9	-	60	0	16	66	2	-	84	0	15	1	5	-	21	0	2	1	1	-	4	169
% Single-Unit Trucks	-	0.8	0.9	0.3	-	0.7	-	1.0	1.1	2.9	-	1.1	-	0.4	0.3	0.3	-	0.4	-	2.0	0.2	1.0	-	0.5	0.7
Articulated Trucks	0	0	7	1	-	8	0	1	16	0	-	17	0	5	0	0	-	5	0	0	0	1	-	1	31
% Articulated Trucks	-	0.0	0.1	0.0	-	0.1	-	0.1	0.3	0.0	-	0.2	-	0.1	0.0	0.0	-	0.1	-	0.0	0.0	1.0	-	0.1	0.1
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	2	0	-	2	0	0	0	0	-	0	2
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.6	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	2	-	-	-	-	-	3	-	-	-	-	-	6	-	-	-	-	-	8	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Kenig Lindgren O'Hara Aboona, Inc.
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018
(847)518-9990

Count Name: 63rd Street with Woodward Avenue
Site Code:
Start Date: 01/21/2017
Page No: 4

Turning Movement Peak Hour Data (12:45 PM)

Start Time	63rd Street Eastbound						63rd Street Westbound						Woodward Avenue Northbound						Woodward Avenue Southbound						
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total
12:45 PM	0	3	159	94	0	256	0	55	205	1	0	261	0	116	12	60	0	188	0	5	20	2	0	27	732
1:00 PM	0	8	196	117	0	321	0	42	195	1	0	238	0	85	9	56	0	150	0	5	6	9	0	20	729
1:15 PM	0	2	178	104	0	284	0	46	202	2	0	250	0	102	10	51	1	163	0	4	16	1	0	21	718
1:30 PM	0	7	175	126	0	308	0	46	222	2	0	270	0	108	8	67	1	183	0	4	13	5	0	22	783
Total	0	20	708	441	0	1169	0	189	824	6	0	1019	0	411	39	234	2	684	0	18	55	17	0	90	2962
Approach %	0.0	1.7	60.6	37.7	-	-	0.0	18.5	80.9	0.6	-	-	0.0	60.1	5.7	34.2	-	-	0.0	20.0	61.1	18.9	-	-	-
Total %	0.0	0.7	23.9	14.9	-	39.5	0.0	6.4	27.8	0.2	-	34.4	0.0	13.9	1.3	7.9	-	23.1	0.0	0.6	1.9	0.6	-	3.0	-
PHF	0.000	0.625	0.903	0.875	-	0.910	0.000	0.859	0.928	0.750	-	0.944	0.000	0.886	0.813	0.873	-	0.910	0.000	0.900	0.688	0.472	-	0.833	0.946
Lights	0	19	701	439	-	1159	0	188	810	6	-	1004	0	411	38	234	-	683	0	17	55	17	-	89	2935
% Lights	-	95.0	99.0	99.5	-	99.1	-	99.5	98.3	100.0	-	98.5	-	100.0	97.4	100.0	-	99.9	-	94.4	100.0	100.0	-	98.9	99.1
Buses	0	0	4	0	-	4	0	1	8	0	-	9	0	0	0	0	-	0	0	1	0	0	-	1	14
% Buses	-	0.0	0.6	0.0	-	0.3	-	0.5	1.0	0.0	-	0.9	-	0.0	0.0	0.0	-	0.0	-	5.6	0.0	0.0	-	1.1	0.5
Single-Unit Trucks	0	1	3	2	-	6	0	0	5	0	-	5	0	0	0	0	-	0	0	0	0	0	-	0	11
% Single-Unit Trucks	-	5.0	0.4	0.5	-	0.5	-	0.0	0.6	0.0	-	0.5	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.4
Articulated Trucks	0	0	0	0	-	0	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	1
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.1	0.0	-	0.1	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	0	-	1	0	0	0	0	-	0	1
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	2.6	0.0	-	0.1	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	2	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-



Kenig Lindgren O'Hara Aboona, Inc.
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018
(847)518-9990

Count Name: 63rd Street with Woodward Avenue
Site Code:
Start Date: 01/21/2017
Page No: 6

Turning Movement Peak Hour Data (7:15 AM)

Start Time	63rd Street Eastbound						63rd Street Westbound						Woodward Avenue Northbound						Woodward Avenue Southbound						
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	Int. Total
7:15 AM	0	3	191	40	0	234	0	15	193	0	0	208	0	218	17	72	0	307	0	3	5	2	0	10	759
7:30 AM	0	7	194	55	0	256	0	26	232	2	0	260	0	213	26	85	0	324	0	3	14	4	0	21	861
7:45 AM	0	9	204	43	0	256	0	43	235	9	0	287	0	212	29	97	0	338	0	9	2	2	0	13	894
8:00 AM	0	4	175	48	0	227	0	22	200	6	0	228	0	169	9	74	0	252	0	6	16	3	1	25	732
Total	0	23	764	186	0	973	0	106	860	17	0	983	0	812	81	328	0	1221	0	21	37	11	1	69	3246
Approach %	0.0	2.4	78.5	19.1	-	-	0.0	10.8	87.5	1.7	-	-	0.0	66.5	6.6	26.9	-	-	0.0	30.4	53.6	15.9	-	-	-
Total %	0.0	0.7	23.5	5.7	-	30.0	0.0	3.3	26.5	0.5	-	30.3	0.0	25.0	2.5	10.1	-	37.6	0.0	0.6	1.1	0.3	-	2.1	-
PHF	0.000	0.639	0.936	0.845	-	0.950	0.000	0.616	0.915	0.472	-	0.856	0.000	0.931	0.698	0.845	-	0.903	0.000	0.583	0.578	0.688	-	0.690	0.908
Lights	0	23	734	176	0	933	0	104	826	16	0	946	0	798	77	315	0	1190	0	20	37	11	0	68	3137
% Lights	-	100.0	96.1	94.6	-	95.9	-	98.1	96.0	94.1	-	96.2	-	98.3	95.1	96.0	-	97.5	-	95.2	100.0	100.0	-	98.6	96.6
Buses	0	0	21	8	0	29	0	2	12	0	0	14	0	10	4	12	0	26	0	1	0	0	0	1	70
% Buses	-	0.0	2.7	4.3	-	3.0	-	1.9	1.4	0.0	-	1.4	-	1.2	4.9	3.7	-	2.1	-	4.8	0.0	0.0	-	1.4	2.2
Single-Unit Trucks	0	0	7	2	0	9	0	0	15	1	0	16	0	4	0	1	0	5	0	0	0	0	0	0	30
% Single-Unit Trucks	-	0.0	0.9	1.1	-	0.9	-	0.0	1.7	5.9	-	1.6	-	0.5	0.0	0.3	-	0.4	-	0.0	0.0	0.0	-	0.0	0.9
Articulated Trucks	0	0	2	0	0	2	0	0	7	0	0	7	0	0	0	0	0	0	0	0	0	0	0	0	9
% Articulated Trucks	-	0.0	0.3	0.0	-	0.2	-	0.0	0.8	0.0	-	0.7	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.3
Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	-	-	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-

Level of Service Criteria

- LEVEL OF SERVICE CRITERIA

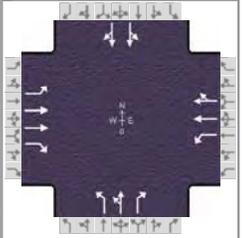
Signalized Intersections		
Level of Service	Interpretation	Average Control Delay (seconds per vehicle)
A	Favorable progression. Most vehicles arrive during the green indication and travel through the intersection without stopping.	≤10
B	Good progression, with more vehicles stopping than for Level of Service A.	>10 - 20
C	Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear. Number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.	>20 - 35
D	The volume-to-capacity ratio is high and either progression is ineffective or the cycle length is too long. Many vehicles stop and individual cycle failures are noticeable.	>35 - 55
E	Progression is unfavorable. The volume-to-capacity ratio is high and the cycle length is long. Individual cycle failures are frequent.	>55 - 80
F	The volume-to-capacity ratio is very high, progression is very poor and the cycle length is long. Most cycles fail to clear the queue.	>80.0
Unsignalized Intersections		
Level of Service	Average Total Delay (SEC/VEH)	
A	0 - 10	
B	> 10 - 15	
C	> 15 - 25	
D	> 25 - 35	
E	> 35 - 50	
F	> 50	

Source: *Highway Capacity Manual, 2010.*

Capacity Analysis Sheets

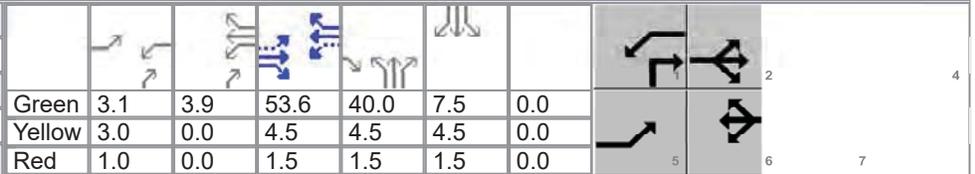
HCS 2010 Signalized Intersection Input Data

General Information				Intersection Information	
Agency	KLOA, Inc.			Duration, h	0.25
Analyst	NJB	Analysis Date	1/24/2017	Area Type	Other
Jurisdiction	DuPage County	Time Period	AM	PHF	0.91
Urban Street	63rd Street	Analysis Year	2017	Analysis Period	1 > 7:00
Intersection	63rd Street with Woodw...	File Name	63rd and Woodward AMEX.xus		
Project Description	Existing AM Peak Hour				



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	23	764	186	106	860	17	812	81	328	21	37	11

Signal Information			
Cycle, s	130.0	Reference Phase	6
Offset, s	0	Reference Point	Begin
Uncoordinated	No	Simult. Gap E/W	On
Force Mode	Fixed	Simult. Gap N/S	On



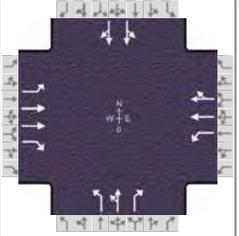
Traffic Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	23	764	186	106	860	17	812	81	328	21	37	11
Initial Queue (Q _b), veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Base Saturation Flow Rate (s ₀), veh/h	1900	2000	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Parking (N _m), man/h	None			None			None			None		
Heavy Vehicles (P _{HV}), %	0	4	5	2	4		2	5	4		0	
Ped / Bike / RTOR, /h	0	0	0	0	0	0	0	0	0	0	0	0
Buses (N _b), buses/h	0	0	0	0	0	0	0	0	0	0	0	0
Arrival Type (AT)	3	4	3	3	4	3	3	3	3	3	3	3
Upstream Filtering (I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Width (W), ft	12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0		12.0	
Turn Bay Length, ft	125	0	350	340	0		305	0	180		0	
Grade (P _g), %		0			0			0			0	
Speed Limit, mi/h	40	40	40	40	40	40	30	30	30	25	25	25

Phase Information	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Maximum Green (G _{max}) or Phase Split, s	13.0	52.0	16.0	55.0	48.0	48.0		14.0
Yellow Change Interval (Y), s	3.0	4.5	3.0	4.5	4.5	4.5		4.5
Red Clearance Interval (R _c), s	1.0	1.5	1.0	1.5	1.5	1.5		1.5
Minimum Green (G _{min}), s	3	15	3	15	3	8	3	8
Start-Up Lost Time (lt), s	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Extension of Effective Green (e), s	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Passage (PT), s	3.0	7.0	3.0	7.0	3.0	4.0	3.0	4.0
Recall Mode	Off	Min	Off	Min	Off	Off	Off	Off
Dual Entry	Yes	Yes	Yes	Yes	No	Yes	No	Yes
Walk (Walk), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Pedestrian Clearance Time (PC), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Multimodal Information	EB			WB			NB			SB		
85th % Speed / Rest in Walk / Corner Radius	0	No	25									
Walkway / Crosswalk Width / Length, ft	9.0	12	0	9.0	12	0	9.0	12	0	9.0	12	0
Street Width / Island / Curb	0	0	No									
Width Outside / Bike Lane / Shoulder, ft	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0
Pedestrian Signal / Occupied Parking	No	0.50										

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB	Analysis Date	1/24/2017	Area Type	Other		
Jurisdiction	DuPage County	Time Period	AM	PHF	0.91		
Urban Street	63rd Street	Analysis Year	2017	Analysis Period	1 > 7:00		
Intersection	63rd Street with Woodw...	File Name	63rd and Woodward AMEX.xus				
Project Description	Existing AM Peak Hour						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	23	764	186	106	860	17	812	81	328	21	37	11

Signal Information				Signal Timing (s)									Signal Phases				
Cycle, s	130.0	Reference Phase	6														
Offset, s	0	Reference Point	Begin	Green	3.1	3.9	53.6	40.0	7.5	0.0							
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.0	0.0	4.5	4.5	4.5	0.0							
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	0.0	1.5	1.5	1.5	0.0							

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase	5	2	1	6		8		4
Case Number	1.1	3.0	1.1	4.0		9.0		12.0
Phase Duration, s	7.1	59.6	10.9	63.5		46.0		13.5
Change Period, (Y+R _c), s	4.0	6.0	4.0	6.0		6.0		6.0
Max Allow Headway (MAH), s	4.0	0.0	4.0	0.0		5.1		5.2
Queue Clearance Time (g _s), s	3.0		6.8			36.4		4.7
Green Extension Time (g _e), s	0.0	0.0	0.2	0.0		3.6		0.1
Phase Call Probability	1.00		1.00			1.00		0.94
Max Out Probability	0.00		0.03			0.97		0.90

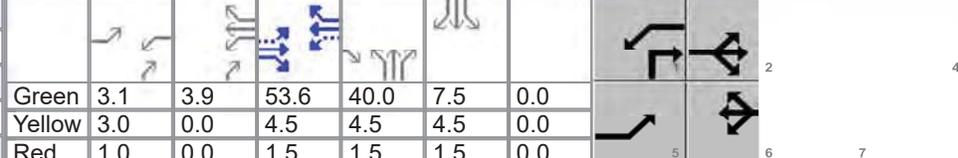
Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Assigned Movement	5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow Rate (v), veh/h	25	840	204	116	484	480	491	491	360	40		36
Adjusted Saturation Flow Rate (s), veh/h/ln	1810	1831	1533	1774	1827	1814	1774	1780	1548	1846		1792
Queue Service Time (g _s), s	1.0	19.3	5.6	4.8	21.8	22.0	34.4	34.2	25.2	2.7		2.5
Cycle Queue Clearance Time (g _c), s	1.0	19.3	5.6	4.8	21.8	22.0	34.4	34.2	25.2	2.7		2.5
Green Ratio (g/C)	0.44	0.41	0.72	0.48	0.44	0.44	0.31	0.31	0.36	0.06		0.06
Capacity (c), veh/h	250	1509	1104	322	807	802	546	548	559	106		103
Volume-to-Capacity Ratio (X)	0.101	0.556	0.185	0.362	0.599	0.599	0.899	0.895	0.645	0.374		0.349
Back of Queue (Q), ft/ln (95 th percentile)	20.2	307.6	80	90.7	342.4	334.2	621.3	632.7	388.6	64		55.6
Back of Queue (Q), veh/ln (95 th percentile)	0.8	11.9	3.1	3.6	13.3	13.4	24.5	24.3	15.1	2.5		2.2
Queue Storage Ratio (RQ) (95 th percentile)	0.16	0.00	0.23	0.27	0.00	0.00	2.04	0.00	2.16	0.00		0.00
Uniform Delay (d ₁), s/veh	22.6	21.6	5.9	20.9	19.4	19.7	43.1	43.0	34.6	59.0		58.9
Incremental Delay (d ₂), s/veh	0.2	1.5	0.4	0.7	3.3	3.3	17.1	16.5	2.7	3.1		2.9
Initial Queue Delay (d ₃), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
Control Delay (d), s/veh	22.8	23.0	6.3	21.6	22.7	23.0	60.1	59.5	37.3	62.1		61.8
Level of Service (LOS)	C	C	A	C	C	C	E	E	D	E		E
Approach Delay, s/veh / LOS	19.8		B	22.7		C	53.8		D	61.9		E
Intersection Delay, s/veh / LOS	34.4						C					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.8	C	2.3	B	2.9	C	3.0	C
Bicycle LOS Score / LOS	1.4	A	1.4	A	2.7	B	0.6	A

HCS 2010 Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB	Analysis Date	1/24/2017		Area Type	Other	
Jurisdiction	DuPage County		Time Period	AM	PHF	0.91	
Urban Street	63rd Street		Analysis Year	2017	Analysis Period	1 > 7:00	
Intersection	63rd Street with Woodw...		File Name	63rd and Woodward AMEX.xus			
Project Description	Existing AM Peak Hour						

Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	23	764	186	106	860	17	812	81	328	21	37	11

Signal Information																								
Cycle, s	130.0	Reference Phase	6	Green	3.1	3.9	53.6	40.0	7.5	0.0	Yellow	3.0	0.0	4.5	4.5	4.5	0.0	Red	1.0	0.0	1.5	1.5	1.5	0.0
Offset, s	0	Reference Point	Begin																					
Uncoordinated	No	Simult. Gap E/W	On																					
Force Mode	Fixed	Simult. Gap N/S	On																					

Saturation Flow / Delay	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Heavy Vehicle Adjustment Factor (f_{HV})	1.000	0.962	0.952	0.980	0.962	1.000	0.980	0.952	0.962	0.952	1.000	1.000
Approach Grade Adjustment Factor (f_g)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Parking Activity Adjustment Factor (f_p)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Lane Utilization Adjustment Factor (f_{LU})	1.000	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})	0.952	0.000		0.952	0.000		0.952	0.000		0.000	0.972	
Right-Turn Adjustment Factor (f_{RT})		0.000	0.847		0.993	0.993		0.000	0.847		0.937	0.943
Left-Turn Pedestrian Adjustment Factor (f_{LPB})	1.000			1.000			1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})			1.000			1.000			1.000			1.000
Movement Saturation Flow Rate (s), veh/h	1810	3662	1533	1774	3571	71	1774	1780	1548	1071	1966	601
Proportion of Vehicles Arriving on Green (P)	0.02	0.55	0.41	0.05	0.59	0.44	0.31	0.31	0.31	0.06	0.06	0.06
Incremental Delay Factor (k)	0.11	0.50	0.50	0.11	0.50	0.50	0.40	0.40	0.24	0.15		0.15

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)	4.0	6.0	4.0	6.0		6.0		6.0
Green Ratio (g/C)	0.44	0.41	0.48	0.44		0.31		0.06
Permitted Saturation Flow Rate (s_p), veh/h/ln	592	0	652	0		1774		0
Shared Saturation Flow Rate (s_{sh}), veh/h/ln								
Permitted Effective Green Time (g_p), s	53.6	0.0	55.5	0.0		0.0		0.0
Permitted Service Time (g_u), s	33.4	0.0	34.2	0.0		0.0		0.0
Permitted Queue Service Time (g_{ps}), s	0.9		4.6					
Time to First Blockage (g_t), s	0.0	0.0	0.0	0.0		0.0		0.0
Queue Service Time Before Blockage (g_{ts}), s								
Protected Right Saturation Flow (s_R), veh/h/ln			1533			1548		
Protected Right Effective Green Time (g_R), s			40.0			6.9		

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	2.107	0.00	1.557	0.00	2.107	0.00	2.224	0.00				
Pedestrian F_s / F_{delay}	0.000	0.125	0.000	0.121	0.000	0.172	0.000	0.163				
Pedestrian M_{corner} / M_{cw}												
Bicycle c_b / d_b	824.44	22.46	883.88	20.24		72.19	115.11	57.73				
Bicycle F_w / F_v	-3.64	0.88	-3.64	0.89	-3.64	2.21	-3.64	0.06				

--- Messages ---

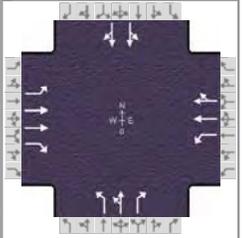
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

WARNING: The shared-plus-exclusive turn lane solution is an approximation of the HCM method, because more than three lane groups cannot be accommodated. Input data for Percent Turns in Shared Lane are used to specify proportion of turning vehicles in the shared lane.

--- Comments ---

HCS 2010 Signalized Intersection Input Data

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB	Analysis Date	1/24/2017	Area Type	Other		
Jurisdiction	DuPage County	Time Period	PM	PHF	0.97		
Urban Street	63rd Street	Analysis Year	2017	Analysis Period	1 > 7:00		
Intersection	63rd Street with Woodw...	File Name	63rd and Woodward PMEX.xus				
Project Description	Existing PM Peak Hour						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	24	956	706	289	828	5	336	49	191	22	140	16

Signal Information				Signal Timing (s)									Signal Phases			
Cycle, s	130.0	Reference Phase	2													
Offset, s	0	Reference Point	Begin	Green	3.0	5.5	67.7	18.6	9.2	0.0						
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.0	3.0	4.5	4.5	4.5	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	1.5	1.5	1.5	0.0						

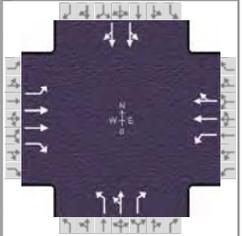
Traffic Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	24	956	706	289	828	5	336	49	191	22	140	16
Initial Queue (Q _b), veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Base Saturation Flow Rate (s ₀), veh/h	1900	2000	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Parking (N _m), man/h		None			None			None			None	
Heavy Vehicles (P _{HV}), %	0	1	0	1	1		1	2	2		0	
Ped / Bike / RTOR, /h	0	0	0	0	0	0	0	0	0	0	0	0
Buses (N _b), buses/h	0	0	0	0	0	0	0	0	0	0	0	0
Arrival Type (AT)	3	4	3	3	4	3	3	3	3	3	3	3
Upstream Filtering (I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Width (W), ft	12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0		12.0	
Turn Bay Length, ft	125	0	350	340	0		305	0	180		0	
Grade (P _g), %		0			0			0			0	
Speed Limit, mi/h	40	40	40	40	40	40	30	30	30	25	25	25

Phase Information	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Maximum Green (G _{max}) or Phase Split, s	13.0	48.0	30.0	65.0	31.0	31.0		21.0
Yellow Change Interval (Y), s	3.0	4.5	3.0	4.5	4.5	4.5		4.5
Red Clearance Interval (R _c), s	1.0	1.5	1.0	1.5	1.5	1.5		1.5
Minimum Green (G _{min}), s	3	15	3	15	3	8	3	8
Start-Up Lost Time (lt), s	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Extension of Effective Green (e), s	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Passage (PT), s	3.0	7.0	3.0	7.0	3.0	4.0	3.0	4.0
Recall Mode	Off	Min	Off	Min	Off	Off	Off	Off
Dual Entry	Yes	Yes	Yes	Yes	No	Yes	No	Yes
Walk (Walk), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Pedestrian Clearance Time (PC), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Multimodal Information	EB			WB			NB			SB		
85th % Speed / Rest in Walk / Corner Radius	0	No	25									
Walkway / Crosswalk Width / Length, ft	9.0	12	0	9.0	12	0	9.0	12	0	9.0	12	0
Street Width / Island / Curb	0	0	No									
Width Outside / Bike Lane / Shoulder, ft	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0
Pedestrian Signal / Occupied Parking	No	0.50										

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB		Analysis Date	1/24/2017		Area Type	Other
Jurisdiction	DuPage County		Time Period	PM		PHF	0.97
Urban Street	63rd Street		Analysis Year	2017		Analysis Period	1 > 7:00
Intersection	63rd Street with Woodw...		File Name	63rd and Woodward PMEX.xus			
Project Description	Existing PM Peak Hour						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	24	956	706	289	828	5	336	49	191	22	140	16

Signal Information				Signal Timing (s)									Signal Phases			
Cycle, s	130.0	Reference Phase	2													
Offset, s	0	Reference Point	Begin	Green	3.0	5.5	67.7	18.6	9.2	0.0						
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.0	3.0	4.5	4.5	4.5	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	1.5	1.5	1.5	0.0						

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase	5	2	1	6		8		4
Case Number	1.1	3.0	1.1	4.0		9.0		12.0
Phase Duration, s	7.0	73.7	16.5	83.2		24.6		15.2
Change Period, (Y+R _c), s	4.0	6.0	4.0	6.0		6.0		6.0
Max Allow Headway (MAH), s	4.0	0.0	4.0	0.0		5.2		5.1
Queue Clearance Time (g _s), s	2.8		11.5			16.3		8.5
Green Extension Time (g _e), s	0.0	0.0	1.0	0.0		2.2		0.7
Phase Call Probability	1.00		1.00			1.00		1.00
Max Out Probability	0.00		0.00			0.45		0.01

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow Rate (v), veh/h	25	986	728	298	430	429	191	206	197	96		87
Adjusted Saturation Flow Rate (s), veh/h/ln	1810	1885	1610	1792	1881	1877	1792	1809	1579	1878		1837
Queue Service Time (g _s), s	0.8	16.0	36.1	9.5	8.9	9.0	13.3	14.3	14.1	6.5		6.0
Cycle Queue Clearance Time (g _c), s	0.8	16.0	36.1	9.5	8.9	9.0	13.3	14.3	14.1	6.5		6.0
Green Ratio (g/C)	0.54	0.52	0.66	0.63	0.59	0.59	0.14	0.14	0.24	0.07		0.07
Capacity (c), veh/h	430	1962	1068	456	1117	1115	256	259	378	133		130
Volume-to-Capacity Ratio (X)	0.058	0.502	0.681	0.653	0.385	0.385	0.744	0.798	0.521	0.724		0.669
Back of Queue (Q), ft/ln (95 th percentile)	15.1	236.2	464.8	169.8	142.4	142.5	269.4	298.4	244.1	164.4		140.3
Back of Queue (Q), veh/ln (95 th percentile)	0.6	9.4	18.6	6.7	5.7	5.7	10.7	11.7	9.6	6.3		5.6
Queue Storage Ratio (RQ) (95 th percentile)	0.12	0.00	1.33	0.50	0.00	0.00	0.88	0.00	1.36	0.00		0.00
Uniform Delay (d ₁), s/veh	13.7	12.0	13.4	13.1	6.4	6.5	53.4	53.9	43.0	59.1		58.9
Incremental Delay (d ₂), s/veh	0.1	0.9	3.5	1.6	1.0	1.0	7.3	10.7	1.6	10.1		8.1
Initial Queue Delay (d ₃), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
Control Delay (d), s/veh	13.8	12.9	17.0	14.7	7.4	7.5	60.7	64.6	44.5	69.3		67.0
Level of Service (LOS)	B	B	B	B	A	A	E	E	D	E		E
Approach Delay, s/veh / LOS	14.6		B	9.3		A	56.7		E	68.2		E
Intersection Delay, s/veh / LOS	22.4						C					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.8	C	2.3	B	2.9	C	3.0	C
Bicycle LOS Score / LOS	1.9	A	1.4	A	1.5	A	0.6	A

HCS 2010 Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB	Analysis Date	1/24/2017	Area Type	Other		
Jurisdiction	DuPage County	Time Period	PM	PHF	0.97		
Urban Street	63rd Street	Analysis Year	2017	Analysis Period	1 > 7:00		
Intersection	63rd Street with Woodw...	File Name	63rd and Woodward PMEX.xus				
Project Description	Existing PM Peak Hour						

Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	24	956	706	289	828	5	336	49	191	22	140	16

Signal Information													
Cycle, s	130.0	Reference Phase	2										
Offset, s	0	Reference Point	Begin										
Uncoordinated	No	Simult. Gap E/W	On	Green	3.0	5.5	67.7	18.6	9.2	0.0			
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.0	3.0	4.5	4.5	4.5	0.0			
				Red	1.0	1.0	1.5	1.5	1.5	0.0			

Saturation Flow / Delay	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Heavy Vehicle Adjustment Factor (f_{HV})	1.000	0.990	1.000	0.990	0.990	1.000	0.990	0.980	0.980	0.952	1.000	1.000
Approach Grade Adjustment Factor (f_g)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Parking Activity Adjustment Factor (f_p)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Lane Utilization Adjustment Factor (f_{LU})	1.000	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})	0.952	0.000		0.952	0.000		0.952	0.000		0.000	0.988	
Right-Turn Adjustment Factor (f_{RT})		0.000	0.847		0.998	0.998		0.000	0.847		0.966	0.967
Left-Turn Pedestrian Adjustment Factor (f_{LPB})	1.000			1.000			1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})			1.000			1.000			1.000			1.000
Movement Saturation Flow Rate (s), veh/h	1810	3770	1610	1792	3736	23	1792	1809	1579	442	2926	348
Proportion of Vehicles Arriving on Green (P)	0.02	0.69	0.52	0.10	0.79	0.59	0.14	0.14	0.14	0.07	0.07	0.07
Incremental Delay Factor (k)	0.11	0.50	0.50	0.11	0.50	0.50	0.19	0.22	0.15	0.15		0.15

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)	4.0	6.0	4.0	6.0		6.0		6.0
Green Ratio (g/C)	0.54	0.52	0.63	0.59		0.14		0.07
Permitted Saturation Flow Rate (s_p), veh/h/ln	654	0	574	0		1792		0
Shared Saturation Flow Rate (s_{sh}), veh/h/ln								
Permitted Effective Green Time (g_p), s	67.7	0.0	69.7	0.0		0.0		0.0
Permitted Service Time (g_u), s	66.2	0.0	51.7	0.0		0.0		0.0
Permitted Queue Service Time (g_{ps}), s	0.1		19.4					
Time to First Blockage (g_t), s	0.0	0.0	0.0	0.0		0.0		0.0
Queue Service Time Before Blockage (g_{ts}), s								
Protected Right Saturation Flow (s_R), veh/h/ln		1610				1579		
Protected Right Effective Green Time (g_R), s		18.6				12.5		

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	2.107	0.00	1.557	0.00	2.107	0.00	2.224	0.00				
Pedestrian F_s / F_{delay}	0.000	0.108	0.000	0.095	0.000	0.172	0.000	0.161				
Pedestrian M_{corner} / M_{cw}												
Bicycle c_b / d_b	1041.02	14.94	1187.69	10.72		72.19	141.77	56.11				
Bicycle F_w / F_v	-3.64	1.43	-3.64	0.95	-3.64	0.98	-3.64	0.15				

--- Messages ---

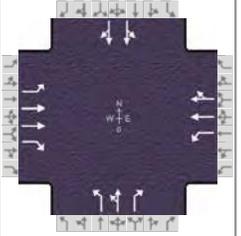
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

WARNING: The shared-plus-exclusive turn lane solution is an approximation of the HCM method, because more than three lane groups cannot be accommodated. Input data for Percent Turns in Shared Lane are used to specify proportion of turning vehicles in the shared lane.

--- Comments ---

HCS 2010 Signalized Intersection Input Data

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB		Analysis Date	1/24/2017		Area Type	Other
Jurisdiction	DuPage County		Time Period	SAT Midday		PHF	0.95
Urban Street	63rd Street		Analysis Year	2017		Analysis Period	1 > 7:00
Intersection	63rd Street with Woodw...		File Name	63rd and Woodward SATEX.xus			
Project Description	Existing SAT Midday Peak Hour						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	20	708	441	189	824	6	411	39	234	18	55	17

Signal Information				Signal Timing (s)									Signal Phases			
Cycle, s	110.0	Reference Phase	2													
Offset, s	0	Reference Point	Begin	Green	3.0	1.5	52.1	19.8	7.6	0.0						
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.0	3.0	4.5	4.5	4.5	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	1.5	1.5	1.5	0.0						

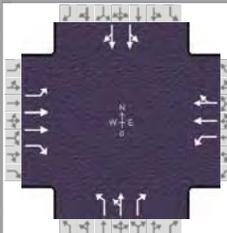
Traffic Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	20	708	441	189	824	6	411	39	234	18	55	17
Initial Queue (Q _b), veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Base Saturation Flow Rate (s ₀), veh/h	1900	2000	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Parking (N _m), man/h		None			None			None			None	
Heavy Vehicles (P _{HV}), %	5	1	1	1	2		0	3	0		0	
Ped / Bike / RTOR, /h	0	0	0	0	0	0	0	0	0	0	0	0
Buses (N _b), buses/h	0	0	0	0	0	0	0	0	0	0	0	0
Arrival Type (AT)	3	4	3	3	4	3	3	3	3	3	3	3
Upstream Filtering (I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Width (W), ft	12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0		12.0	
Turn Bay Length, ft	125	0	350	340	0		305	0	180		0	
Grade (P _g), %		0			0			0			0	
Speed Limit, mi/h	40	40	40	40	40	40	30	30	30	25	25	25

Phase Information	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
	Maximum Green (G _{max}) or Phase Split, s	13.0	39.0	19.0	45.0	33.0	33.0	
Yellow Change Interval (Y), s	3.0	4.5	3.0	4.5	4.5	4.5		4.5
Red Clearance Interval (R _c), s	1.0	1.5	1.0	1.5	1.5	1.5		1.5
Minimum Green (G _{min}), s	3	15	3	15	3	8	3	8
Start-Up Lost Time (lt), s	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Extension of Effective Green (e), s	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Passage (PT), s	3.0	7.0	3.0	7.0	3.0	4.0	3.0	4.0
Recall Mode	Off	Min	Off	Min	Off	Off	Off	Off
Dual Entry	Yes	Yes	Yes	Yes	No	Yes	No	Yes
Walk (Walk), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Pedestrian Clearance Time (PC), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Multimodal Information	EB			WB			NB			SB		
	85th % Speed / Rest in Walk / Corner Radius	0	No	25	0	No	25	0	No	25	0	No
Walkway / Crosswalk Width / Length, ft	9.0	12	0	9.0	12	0	9.0	12	0	9.0	12	0
Street Width / Island / Curb	0	0	No	0	0	No	0	0	No	0	0	No
Width Outside / Bike Lane / Shoulder, ft	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0
Pedestrian Signal / Occupied Parking	No	0.50		No	0.50		No	0.50		No	0.50	

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB	Analysis Date	1/24/2017	Area Type	Other		
Jurisdiction	DuPage County	Time Period	SAT Midday	PHF	0.95		
Urban Street	63rd Street	Analysis Year	2017	Analysis Period	1 > 7:00		
Intersection	63rd Street with Woodw...	File Name	63rd and Woodward SATEX.xus				
Project Description	Existing SAT Midday Peak Hour						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	20	708	441	189	824	6	411	39	234	18	55	17

Signal Information				Signal Timing (s)									Signal Phases			
Cycle, s	110.0	Reference Phase	2													
Offset, s	0	Reference Point	Begin	Green	3.0	1.5	52.1	19.8	7.6	0.0						
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.0	3.0	4.5	4.5	4.5	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	1.5	1.5	1.5	0.0						

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase	5	2	1	6		8		4
Case Number	1.1	3.0	1.1	4.0		9.0		12.0
Phase Duration, s	7.0	58.1	12.5	63.6		25.8		13.6
Change Period, ($Y+R_c$), s	4.0	6.0	4.0	6.0		6.0		6.0
Max Allow Headway (MAH), s	4.0	0.0	4.0	0.0		5.2		5.2
Queue Clearance Time (g_s), s	2.7		7.9			16.8		4.8
Green Extension Time (g_e), s	0.0	0.0	0.6	0.0		3.1		0.3
Phase Call Probability	1.00		1.00			1.00		0.94
Max Out Probability	0.00		0.00			0.39		0.00

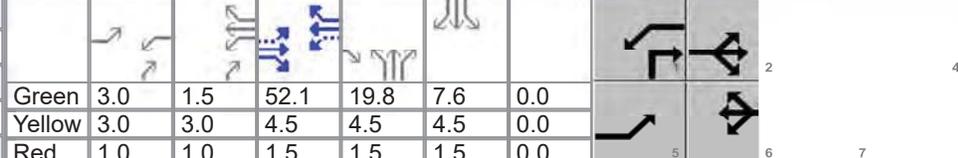
Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow Rate (v), veh/h	21	745	464	199	437	436	238	236	246	50		45
Adjusted Saturation Flow Rate (s), veh/h/ln	1723	1885	1594	1792	1863	1858	1810	1816	1610	1865		1773
Queue Service Time (g_s), s	0.7	10.9	15.6	5.9	11.3	11.4	13.7	13.5	14.8	2.8		2.7
Cycle Queue Clearance Time (g_c), s	0.7	10.9	15.6	5.9	11.3	11.4	13.7	13.5	14.8	2.8		2.7
Green Ratio (g/C)	0.50	0.47	0.65	0.57	0.52	0.52	0.18	0.18	0.26	0.07		0.07
Capacity (c), veh/h	359	1787	1043	474	976	973	326	327	414	128		122
Volume-to-Capacity Ratio (X)	0.059	0.417	0.445	0.420	0.448	0.448	0.730	0.721	0.595	0.389		0.369
Back of Queue (Q), ft/ln (95 th percentile)	12.1	182.5	223.2	103.1	184.8	183.2	268.1	271.3	249.7	66.9		57.5
Back of Queue (Q), veh/ln (95 th percentile)	0.5	7.2	8.9	4.1	7.3	7.3	10.7	10.6	10.0	2.6		2.3
Queue Storage Ratio (RQ) (95 th percentile)	0.10	0.00	0.64	0.30	0.00	0.00	0.88	0.00	1.39	0.00		0.00
Uniform Delay (d_1), s/veh	14.1	12.6	9.3	12.5	9.6	9.7	42.6	42.5	35.8	49.0		48.9
Incremental Delay (d_2), s/veh	0.1	0.7	1.4	0.6	1.5	1.5	5.1	4.8	1.9	2.7		2.6
Initial Queue Delay (d_3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
Control Delay (d), s/veh	14.2	13.4	10.7	13.1	11.1	11.2	47.7	47.3	37.8	51.7		51.6
Level of Service (LOS)	B	B	B	B	B	B	D	D	D	D		D
Approach Delay, s/veh / LOS	12.4		B	11.5		B	44.2		D	51.7		D
Intersection Delay, s/veh / LOS	20.6						C					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.8	C	2.3	B	2.9	C	3.0	C
Bicycle LOS Score / LOS	1.5	A	1.4	A	1.7	A	0.6	A

HCS 2010 Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB	Analysis Date	1/24/2017	Area Type	Other		
Jurisdiction	DuPage County	Time Period	SAT Midday	PHF	0.95		
Urban Street	63rd Street	Analysis Year	2017	Analysis Period	1 > 7:00		
Intersection	63rd Street with Woodw...	File Name	63rd and Woodward SATEX.xus				
Project Description	Existing SAT Midday Peak Hour						

Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	20	708	441	189	824	6	411	39	234	18	55	17

Signal Information													
Cycle, s	110.0	Reference Phase	2										
Offset, s	0	Reference Point	Begin										
Uncoordinated	No	Simult. Gap E/W	On	Green	3.0	1.5	52.1	19.8	7.6	0.0			
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.0	3.0	4.5	4.5	4.5	0.0			
				Red	1.0	1.0	1.5	1.5	1.5	0.0			

Saturation Flow / Delay	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Heavy Vehicle Adjustment Factor (f_{HV})	0.952	0.990	0.990	0.990	0.980	1.000	1.000	0.971	1.000	0.943	1.000	1.000
Approach Grade Adjustment Factor (f_g)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Parking Activity Adjustment Factor (f_p)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Lane Utilization Adjustment Factor (f_{LU})	1.000	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})	0.952	0.000		0.952	0.000		0.952	0.000		0.000	0.981	
Right-Turn Adjustment Factor (f_{RT})		0.000	0.847		0.997	0.997		0.000	0.847		0.929	0.933
Left-Turn Pedestrian Adjustment Factor (f_{LPB})	1.000			1.000			1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})			1.000			1.000			1.000			1.000
Movement Saturation Flow Rate (s), veh/h	1723	3770	1594	1792	3694	27	1810	1816	1610	709	2222	706
Proportion of Vehicles Arriving on Green (P)	0.03	0.63	0.47	0.08	0.70	0.52	0.18	0.18	0.18	0.07	0.07	0.07
Incremental Delay Factor (k)	0.11	0.50	0.50	0.11	0.50	0.50	0.18	0.17	0.15	0.15		0.15

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)	4.0	6.0	4.0	6.0		6.0		6.0
Green Ratio (g/C)	0.50	0.47	0.57	0.52		0.18		0.07
Permitted Saturation Flow Rate (s_p), veh/h/ln	614	0	719	0		1810		0
Shared Saturation Flow Rate (s_{sh}), veh/h/ln								
Permitted Effective Green Time (g_p), s	52.1	0.0	54.1	0.0		0.0		0.0
Permitted Service Time (g_u), s	44.2	0.0	41.3	0.0		0.0		0.0
Permitted Queue Service Time (g_{ps}), s	0.3		4.9					
Time to First Blockage (g_t), s	0.0	0.0	0.0	0.0		0.0		0.0
Queue Service Time Before Blockage (g_{ts}), s								
Protected Right Saturation Flow (s_R), veh/h/ln			1594			1610		
Protected Right Effective Green Time (g_R), s			19.8			8.5		

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	2.107	0.00	1.557	0.00	2.107	0.00	2.224	0.00				
Pedestrian F_s / F_{delay}	0.000	0.109	0.000	0.101	0.000	0.166	0.000	0.155				
Pedestrian M_{corner} / M_{cw}												
Bicycle c_b / d_b	947.97	15.22	1047.82	12.47		62.22	137.41	47.70				
Bicycle F_w / F_v	-3.64	1.02	-3.64	0.88	-3.64	1.19	-3.64	0.08				

--- Messages ---

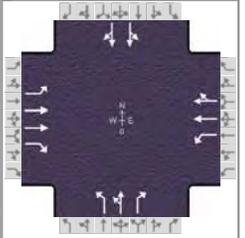
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WARNING: The shared-plus-exclusive turn lane solution is an approximation of the HCM method, because more than three lane groups cannot be accommodated. Input data for Percent Turns in Shared Lane are used to specify proportion of turning vehicles in the shared lane.

--- Comments ---

HCS 2010 Signalized Intersection Input Data

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB		Analysis Date	1/24/2017		Area Type	Other
Jurisdiction	DuPage County		Time Period	AM		PHF	0.91
Urban Street	63rd Street		Analysis Year	2018		Analysis Period	1 > 7:00
Intersection	63rd Street with Woodw...		File Name	63rd and Woodward AMFU.xus			
Project Description	Future AM Peak Hour						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	23	775	188	107	873	17	820	83	331	21	38	11

Signal Information				Signal Timing (s)									Signal Phases			
Cycle, s	130.0	Reference Phase	6													
Offset, s	0	Reference Point	Begin	Green	3.1	3.9	53.3	40.2	7.5	0.0						
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.0	0.0	4.5	4.5	4.5	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	0.0	1.5	1.5	1.5	0.0						

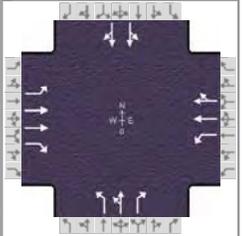
Traffic Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	23	775	188	107	873	17	820	83	331	21	38	11
Initial Queue (Q _b), veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Base Saturation Flow Rate (s ₀), veh/h	1900	2000	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Parking (N _m), man/h		None			None			None			None	
Heavy Vehicles (P _{HV}), %	0	4	5	2	4		2	5	4		0	
Ped / Bike / RTOR, /h	0	0	0	0	0	0	0	0	0	0	0	0
Buses (N _b), buses/h	0	0	0	0	0	0	0	0	0	0	0	0
Arrival Type (AT)	3	4	3	3	4	3	3	3	3	3	3	3
Upstream Filtering (I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Width (W), ft	12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0		12.0	
Turn Bay Length, ft	125	0	350	340	0		305	0	180		0	
Grade (P _g), %		0			0			0			0	
Speed Limit, mi/h	40	40	40	40	40	40	30	30	30	25	25	25

Phase Information	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
	Maximum Green (G _{max}) or Phase Split, s	13.0	52.0	16.0	55.0	48.0	48.0	
Yellow Change Interval (Y), s	3.0	4.5	3.0	4.5	4.5	4.5		4.5
Red Clearance Interval (R _c), s	1.0	1.5	1.0	1.5	1.5	1.5		1.5
Minimum Green (G _{min}), s	3	15	3	15	3	8	3	8
Start-Up Lost Time (lt), s	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Extension of Effective Green (e), s	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Passage (PT), s	3.0	7.0	3.0	7.0	3.0	4.0	3.0	4.0
Recall Mode	Off	Min	Off	Min	Off	Off	Off	Off
Dual Entry	Yes	Yes	Yes	Yes	No	Yes	No	Yes
Walk (Walk), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Pedestrian Clearance Time (PC), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Multimodal Information	EB			WB			NB			SB		
	85th % Speed / Rest in Walk / Corner Radius	0	No	25	0	No	25	0	No	25	0	No
Walkway / Crosswalk Width / Length, ft	9.0	12	0	9.0	12	0	9.0	12	0	9.0	12	0
Street Width / Island / Curb	0	0	No	0	0	No	0	0	No	0	0	No
Width Outside / Bike Lane / Shoulder, ft	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0
Pedestrian Signal / Occupied Parking	No	0.50		No	0.50		No	0.50		No	0.50	

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB	Analysis Date	1/24/2017	Area Type	Other		
Jurisdiction	DuPage County	Time Period	AM	PHF	0.91		
Urban Street	63rd Street	Analysis Year	2018	Analysis Period	1 > 7:00		
Intersection	63rd Street with Woodw...	File Name	63rd and Woodward AMFU.xus				
Project Description	Future AM Peak Hour						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	23	775	188	107	873	17	820	83	331	21	38	11

Signal Information				Signal Timing (s)									Signal Phases											
Cycle, s	130.0	Reference Phase	6	Green	3.1	3.9	53.3	40.2	7.5	0.0	Yellow	3.0	0.0	4.5	4.5	4.5	0.0	Red	1.0	0.0	1.5	1.5	1.5	0.0
Offset, s	0	Reference Point	Begin										5	6	7									
Uncoordinated	No	Simult. Gap E/W	On										2	4										
Force Mode	Fixed	Simult. Gap N/S	On																					

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase	5	2	1	6		8		4
Case Number	1.1	3.0	1.1	4.0		9.0		12.0
Phase Duration, s	7.1	59.3	11.0	63.2		46.2		13.5
Change Period, (Y+R _c), s	4.0	6.0	4.0	6.0		6.0		6.0
Max Allow Headway (MAH), s	4.0	0.0	4.0	0.0		5.1		5.2
Queue Clearance Time (g _s), s	3.0		6.8			36.8		4.7
Green Extension Time (g _e), s	0.0	0.0	0.2	0.0		3.4		0.1
Phase Call Probability	1.00		1.00			1.00		0.94
Max Out Probability	0.00		0.04			1.00		1.00

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow Rate (v), veh/h	25	852	207	118	491	487	496	497	364	40		37
Adjusted Saturation Flow Rate (s), veh/h/ln	1810	1831	1533	1774	1827	1814	1774	1781	1548	1847		1793
Queue Service Time (g _s), s	1.0	19.9	5.7	4.8	22.5	22.7	34.8	34.7	25.4	2.7		2.5
Cycle Queue Clearance Time (g _c), s	1.0	19.9	5.7	4.8	22.5	22.7	34.8	34.7	25.4	2.7		2.5
Green Ratio (g/C)	0.43	0.41	0.72	0.48	0.44	0.44	0.31	0.31	0.36	0.06		0.06
Capacity (c), veh/h	244	1501	1103	316	804	799	549	551	562	107		104
Volume-to-Capacity Ratio (X)	0.103	0.567	0.187	0.372	0.610	0.610	0.903	0.902	0.647	0.379		0.353
Back of Queue (Q), ft/ln (95 th percentile)	20.2	314.9	81.1	91.9	351	343	629.4	644.5	391.5	64.9		56.4
Back of Queue (Q), veh/ln (95 th percentile)	0.8	12.2	3.1	3.6	13.6	13.7	24.8	24.8	15.2	2.5		2.3
Queue Storage Ratio (RQ) (95 th percentile)	0.16	0.00	0.23	0.27	0.00	0.00	2.06	0.00	2.17	0.00		0.00
Uniform Delay (d ₁), s/veh	22.9	21.9	5.9	21.2	19.7	20.0	43.0	43.0	34.5	59.0		58.9
Incremental Delay (d ₂), s/veh	0.2	1.6	0.4	0.7	3.4	3.5	17.7	17.4	2.8	3.1		2.9
Initial Queue Delay (d ₃), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
Control Delay (d), s/veh	23.1	23.4	6.3	21.9	23.1	23.4	60.7	60.4	37.2	62.1		61.8
Level of Service (LOS)	C	C	A	C	C	C	E	E	D	E		E
Approach Delay, s/veh / LOS	20.2		C	23.1		C	54.3		D	62.0		E
Intersection Delay, s/veh / LOS	34.8						C					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.8	C	2.3	B	2.9	C	3.0	C
Bicycle LOS Score / LOS	1.4	A	1.4	A	2.7	B	0.6	A

HCS 2010 Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB	Analysis Date	1/24/2017	Area Type	Other		
Jurisdiction	DuPage County	Time Period	AM	PHF	0.91		
Urban Street	63rd Street	Analysis Year	2018	Analysis Period	1 > 7:00		
Intersection	63rd Street with Woodw...	File Name	63rd and Woodward AMFU.xus				
Project Description	Future AM Peak Hour						

Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	23	775	188	107	873	17	820	83	331	21	38	11

Signal Information													
Cycle, s	130.0	Reference Phase	6										
Offset, s	0	Reference Point	Begin										
Uncoordinated	No	Simult. Gap E/W	On	Green	3.1	3.9	53.3	40.2	7.5	0.0			
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	3.0	0.0	4.5	4.5	4.5	0.0			
				Red	1.0	0.0	1.5	1.5	1.5	0.0			

Saturation Flow / Delay	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Heavy Vehicle Adjustment Factor (f_{HV})	1.000	0.962	0.952	0.980	0.962	1.000	0.980	0.952	0.962	0.952	1.000	1.000
Approach Grade Adjustment Factor (f_g)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Parking Activity Adjustment Factor (f_p)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Lane Utilization Adjustment Factor (f_{LU})	1.000	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})	0.952	0.000		0.952	0.000		0.952	0.000		0.000	0.972	
Right-Turn Adjustment Factor (f_{RT})		0.000	0.847		0.993	0.993		0.000	0.847		0.938	0.944
Left-Turn Pedestrian Adjustment Factor (f_{LPB})	1.000			1.000			1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})			1.000			1.000			1.000			1.000
Movement Saturation Flow Rate (s), veh/h	1810	3662	1533	1774	3572	70	1774	1781	1548	1056	1991	593
Proportion of Vehicles Arriving on Green (P)	0.02	0.55	0.41	0.05	0.59	0.44	0.31	0.31	0.31	0.06	0.06	0.06
Incremental Delay Factor (k)	0.11	0.50	0.50	0.11	0.50	0.50	0.41	0.41	0.24	0.15		0.15

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)	4.0	6.0	4.0	6.0		6.0		6.0
Green Ratio (g/C)	0.43	0.41	0.48	0.44		0.31		0.06
Permitted Saturation Flow Rate (s_p), veh/h/ln	584	0	645	0		1774		0
Shared Saturation Flow Rate (s_{sh}), veh/h/ln								
Permitted Effective Green Time (g_p), s	53.3	0.0	55.2	0.0		0.0		0.0
Permitted Service Time (g_u), s	32.6	0.0	33.4	0.0		0.0		0.0
Permitted Queue Service Time (g_{ps}), s	0.9		4.9					
Time to First Blockage (g_t), s	0.0	0.0	0.0	0.0		0.0		0.0
Queue Service Time Before Blockage (g_{ts}), s								
Protected Right Saturation Flow (s_R), veh/h/ln			1533			1548		
Protected Right Effective Green Time (g_R), s			40.2			7.0		

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	2.107	0.00	1.557	0.00	2.107	0.00	2.224	0.00				
Pedestrian F_s / F_{delay}	0.000	0.125	0.000	0.121	0.000	0.172	0.000	0.163				
Pedestrian M_{corner} / M_{cw}												
Bicycle c_b / d_b	820.05	22.62	880.32	20.37		72.19	115.42	57.71				
Bicycle F_w / F_v	-3.64	0.89	-3.64	0.90	-3.64	2.24	-3.64	0.06				

--- Messages ---

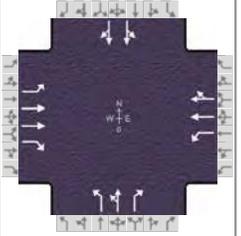
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

WARNING: The shared-plus-exclusive turn lane solution is an approximation of the HCM method, because more than three lane groups cannot be accommodated. Input data for Percent Turns in Shared Lane are used to specify proportion of turning vehicles in the shared lane.

--- Comments ---

HCS 2010 Signalized Intersection Input Data

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB	Analysis Date	1/24/2017	Area Type	Other		
Jurisdiction	DuPage County	Time Period	PM	PHF	0.97		
Urban Street	63rd Street	Analysis Year	2018	Analysis Period	1 > 7:00		
Intersection	63rd Street with Woodw...	File Name	63rd and Woodward PMFU.xus				
Project Description	Future PM Peak Hour						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	24	977	713	292	847	5	339	52	193	22	143	16

Signal Information				Signal Timing (s)									Signal Diagram				
Cycle, s	130.0	Reference Phase	2	Green	3.0	5.7	67.1	18.9	9.3	0.0							
Offset, s	0	Reference Point	Begin	Yellow	3.0	3.0	4.5	4.5	4.5	0.0							
Uncoordinated	No	Simult. Gap E/W	On	Red	1.0	1.0	1.5	1.5	1.5	0.0							
Force Mode	Fixed	Simult. Gap N/S	On														

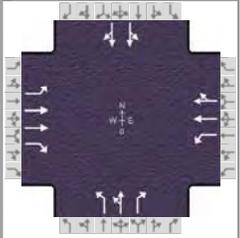
Traffic Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	24	977	713	292	847	5	339	52	193	22	143	16
Initial Queue (Q _b), veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Base Saturation Flow Rate (s ₀), veh/h	1900	2000	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Parking (N _m), man/h		None			None			None			None	
Heavy Vehicles (P _{HV}), %	0	1	0	1	1		1	2	2		0	
Ped / Bike / RTOR, /h	0	0	0	0	0	0	0	0	0	0	0	0
Buses (N _b), buses/h	0	0	0	0	0	0	0	0	0	0	0	0
Arrival Type (AT)	3	4	3	3	4	3	3	3	3	3	3	3
Upstream Filtering (I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Width (W), ft	12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0		12.0	
Turn Bay Length, ft	125	0	350	340	0		305	0	180		0	
Grade (P _g), %		0			0			0			0	
Speed Limit, mi/h	40	40	40	40	40	40	30	30	30	25	25	25

Phase Information	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Maximum Green (G _{max}) or Phase Split, s	13.0	48.0	30.0	65.0	31.0	31.0		21.0
Yellow Change Interval (Y), s	3.0	4.5	3.0	4.5	4.5	4.5		4.5
Red Clearance Interval (R _c), s	1.0	1.5	1.0	1.5	1.5	1.5		1.5
Minimum Green (G _{min}), s	3	15	3	15	3	8	3	8
Start-Up Lost Time (lt), s	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Extension of Effective Green (e), s	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Passage (PT), s	3.0	7.0	3.0	7.0	3.0	4.0	3.0	4.0
Recall Mode	Off	Min	Off	Min	Off	Off	Off	Off
Dual Entry	Yes	Yes	Yes	Yes	No	Yes	No	Yes
Walk (Walk), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Pedestrian Clearance Time (PC), s	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Multimodal Information	EB			WB			NB			SB		
85th % Speed / Rest in Walk / Corner Radius	0	No	25									
Walkway / Crosswalk Width / Length, ft	9.0	12	0	9.0	12	0	9.0	12	0	9.0	12	0
Street Width / Island / Curb	0	0	No									
Width Outside / Bike Lane / Shoulder, ft	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0
Pedestrian Signal / Occupied Parking	No	0.50										

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB	Analysis Date	1/24/2017	Area Type	Other		
Jurisdiction	DuPage County	Time Period	PM	PHF	0.97		
Urban Street	63rd Street	Analysis Year	2018	Analysis Period	1 > 7:00		
Intersection	63rd Street with Woodw...	File Name	63rd and Woodward PMFU.xus				
Project Description	Future PM Peak Hour						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	24	977	713	292	847	5	339	52	193	22	143	16

Signal Information				Signal Timing (s)									Signal Phases			
Cycle, s	130.0	Reference Phase	2													
Offset, s	0	Reference Point	Begin	Green	3.0	5.7	67.1	18.9	9.3	0.0						
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.0	3.0	4.5	4.5	4.5	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	1.5	1.5	1.5	0.0						

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase	5	2	1	6		8		4
Case Number	1.1	3.0	1.1	4.0		9.0		12.0
Phase Duration, s	7.0	73.1	16.7	82.8		24.9		15.3
Change Period, (Y+R _c), s	4.0	6.0	4.0	6.0		6.0		6.0
Max Allow Headway (MAH), s	4.0	0.0	4.0	0.0		5.2		5.1
Queue Clearance Time (g _s), s	2.8		11.7			16.7		8.6
Green Extension Time (g _e), s	0.0	0.0	1.0	0.0		2.2		0.7
Phase Call Probability	1.00		1.00			1.00		1.00
Max Out Probability	0.00		0.00			0.49		0.02

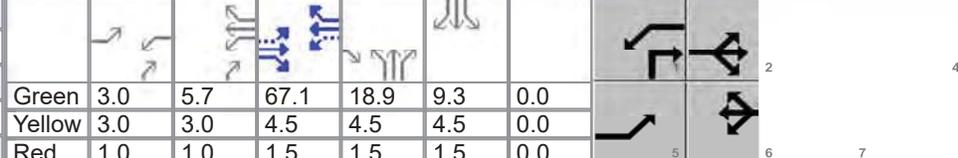
Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow Rate (v), veh/h	25	1007	735	301	440	439	192	211	199	98		89
Adjusted Saturation Flow Rate (s), veh/h/ln	1810	1885	1610	1792	1881	1877	1792	1810	1579	1878		1838
Queue Service Time (g _s), s	0.8	16.8	37.0	9.7	9.4	9.5	13.4	14.7	14.2	6.6		6.1
Cycle Queue Clearance Time (g _c), s	0.8	16.8	37.0	9.7	9.4	9.5	13.4	14.7	14.2	6.6		6.1
Green Ratio (g/C)	0.54	0.52	0.66	0.63	0.59	0.59	0.15	0.15	0.24	0.07		0.07
Capacity (c), veh/h	420	1945	1064	448	1111	1109	260	263	384	135		132
Volume-to-Capacity Ratio (X)	0.059	0.518	0.691	0.672	0.396	0.396	0.739	0.803	0.518	0.728		0.671
Back of Queue (Q), ft/ln (95 th percentile)	15.3	246.2	476.8	174.1	149.3	149.6	270.7	304.5	245	167.2		142.6
Back of Queue (Q), veh/ln (95 th percentile)	0.6	9.8	19.1	6.9	5.9	6.0	10.7	12.0	9.6	6.4		5.7
Queue Storage Ratio (RQ) (95 th percentile)	0.12	0.00	1.36	0.51	0.00	0.00	0.89	0.00	1.36	0.00		0.00
Uniform Delay (d ₁), s/veh	14.0	12.5	13.7	13.7	6.6	6.7	53.2	53.8	42.6	59.1		58.8
Incremental Delay (d ₂), s/veh	0.1	1.0	3.7	1.8	1.1	1.1	7.2	11.2	1.5	10.1		8.1
Initial Queue Delay (d ₃), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
Control Delay (d), s/veh	14.1	13.4	17.4	15.4	7.7	7.8	60.4	65.0	44.1	69.2		67.0
Level of Service (LOS)	B	B	B	B	A	A	E	E	D	E		E
Approach Delay, s/veh / LOS	15.1		B	9.7		A	56.6		E	68.2		E
Intersection Delay, s/veh / LOS	22.7						C					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.8	C	2.3	B	2.9	C	3.0	C
Bicycle LOS Score / LOS	1.9	A	1.5	A	1.5	A	0.6	A

HCS 2010 Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB	Analysis Date	1/24/2017	Area Type	Other		
Jurisdiction	DuPage County	Time Period	PM	PHF	0.97		
Urban Street	63rd Street	Analysis Year	2018	Analysis Period	1 > 7:00		
Intersection	63rd Street with Woodw...	File Name	63rd and Woodward PMFU.xus				
Project Description	Future PM Peak Hour						

Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	24	977	713	292	847	5	339	52	193	22	143	16

Signal Information													
Cycle, s	130.0	Reference Phase	2	Green	3.0	5.7	67.1	18.9	9.3	0.0			
Offset, s	0	Reference Point	Begin	Yellow	3.0	3.0	4.5	4.5	4.5	0.0			
Uncoordinated	No	Simult. Gap E/W	On	Red	1.0	1.0	1.5	1.5	1.5	0.0			
Force Mode	Fixed	Simult. Gap N/S	On										

Saturation Flow / Delay	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Heavy Vehicle Adjustment Factor (f_{HV})	1.000	0.990	1.000	0.990	0.990	1.000	0.990	0.980	0.980	0.952	1.000	1.000
Approach Grade Adjustment Factor (f_g)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Parking Activity Adjustment Factor (f_p)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Lane Utilization Adjustment Factor (f_{LU})	1.000	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})	0.952	0.000		0.952	0.000		0.952	0.000		0.000	0.989	
Right-Turn Adjustment Factor (f_{RT})		0.000	0.847		0.998	0.998		0.000	0.847		0.967	0.968
Left-Turn Pedestrian Adjustment Factor (f_{LPB})	1.000			1.000			1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})			1.000			1.000			1.000			1.000
Movement Saturation Flow Rate (s), veh/h	1810	3770	1610	1792	3736	22	1792	1810	1579	434	2940	342
Proportion of Vehicles Arriving on Green (P)	0.02	0.69	0.52	0.10	0.79	0.59	0.15	0.15	0.15	0.07	0.07	0.07
Incremental Delay Factor (k)	0.11	0.50	0.50	0.11	0.50	0.50	0.19	0.23	0.15	0.15		0.15

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)	4.0	6.0	4.0	6.0		6.0		6.0
Green Ratio (g/C)	0.54	0.52	0.63	0.59		0.15		0.07
Permitted Saturation Flow Rate (s_p), veh/h/ln	642	0	563	0		1792		0
Shared Saturation Flow Rate (s_{sh}), veh/h/ln								
Permitted Effective Green Time (g_p), s	67.1	0.0	69.1	0.0		0.0		0.0
Permitted Service Time (g_u), s	65.3	0.0	50.2	0.0		0.0		0.0
Permitted Queue Service Time (g_{ps}), s	0.1		21.7					
Time to First Blockage (g_t), s	0.0	0.0	0.0	0.0		0.0		0.0
Queue Service Time Before Blockage (g_{ts}), s								
Protected Right Saturation Flow (s_R), veh/h/ln			1610			1579		
Protected Right Effective Green Time (g_R), s			18.9			12.7		

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	2.107	0.00	1.557	0.00	2.107	0.00	2.224	0.00				
Pedestrian F_s / F_{delay}	0.000	0.109	0.000	0.096	0.000	0.172	0.000	0.161				
Pedestrian M_{corner} / M_{cw}												
Bicycle c_b / d_b	1031.61	15.24	1181.49	10.89		72.19	143.49	56.01				
Bicycle F_w / F_v	-3.64	1.46	-3.64	0.97	-3.64	0.99	-3.64	0.15				

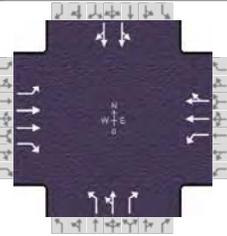
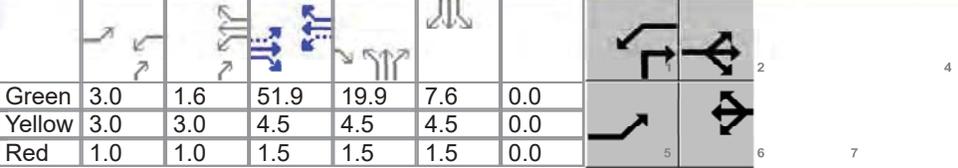
--- Messages ---

WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

WARNING: The shared-plus-exclusive turn lane solution is an approximation of the HCM method, because more than three lane groups cannot be accommodated. Input data for Percent Turns in Shared Lane are used to specify proportion of turning vehicles in the shared lane.

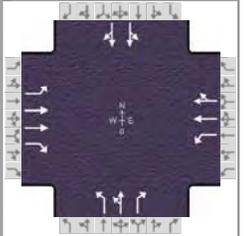
--- Comments ---

HCS 2010 Signalized Intersection Input Data

General Information						Intersection Information																		
Agency	KLOA, Inc.					Duration, h	0.25																	
Analyst	NJB		Analysis Date	1/24/2017		Area Type	Other																	
Jurisdiction	DuPage County		Time Period	SAT Midday		PHF	0.95																	
Urban Street	63rd Street		Analysis Year	2018		Analysis Period	1 > 7:00																	
Intersection	63rd Street with Woodw...		File Name	63rd and Woodward SATFU.xus																				
Project Description	Future SAT Midday Peak Hour																							
Demand Information				EB			WB			NB			SB											
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R									
Demand (v), veh/h				20	724	445	191	841	6	415	41	236	18	57	17									
Signal Information																								
Cycle, s	110.0	Reference Phase	2																					
Offset, s	0	Reference Point	Begin																					
Uncoordinated	No	Simult. Gap E/W	On																					
Force Mode	Fixed	Simult. Gap N/S	On	Green	3.0	1.6	51.9	19.9	7.6	0.0	Yellow	3.0	3.0	4.5	4.5	4.5	0.0	Red	1.0	1.0	1.5	1.5	1.5	0.0
Traffic Information				EB			WB			NB			SB											
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R									
Demand (v), veh/h				20	724	445	191	841	6	415	41	236	18	57	17									
Initial Queue (Q _b), veh/h				0	0	0	0	0	0	0	0	0	0	0	0									
Base Saturation Flow Rate (s ₀), veh/h				1900	2000	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900									
Parking (N _m), man/h				None			None			None			None											
Heavy Vehicles (P _{HV}), %				5	1	1	1	2		0	3	0		0										
Ped / Bike / RTOR, /h				0	0	0	0	0	0	0	0	0	0	0	0									
Buses (N _b), buses/h				0	0	0	0	0	0	0	0	0	0	0										
Arrival Type (AT)				3	4	3	3	4	3	3	3	3	3	3	3									
Upstream Filtering (I)				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00									
Lane Width (W), ft				12.0	12.0	12.0	12.0	12.0		12.0	12.0	12.0		12.0										
Turn Bay Length, ft				125	0	350	340	0		305	0	180		0										
Grade (P _g), %					0			0			0			0										
Speed Limit, mi/h				40	40	40	40	40	40	30	30	30	25	25	25									
Phase Information				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT													
Maximum Green (G _{max}) or Phase Split, s				13.0	39.0	19.0	45.0	33.0	33.0			19.0												
Yellow Change Interval (Y), s				3.0	4.5	3.0	4.5	4.5	4.5			4.5												
Red Clearance Interval (R _c), s				1.0	1.5	1.0	1.5	1.5	1.5			1.5												
Minimum Green (G _{min}), s				3	15	3	15	3	8	3	8													
Start-Up Lost Time (lt), s				2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0													
Extension of Effective Green (e), s				2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0													
Passage (PT), s				3.0	7.0	3.0	7.0	3.0	4.0	3.0	4.0													
Recall Mode				Off	Min	Off	Min	Off	Off	Off	Off													
Dual Entry				Yes	Yes	Yes	Yes	No	Yes	No	Yes													
Walk (Walk), s				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0													
Pedestrian Clearance Time (PC), s				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0													
Multimodal Information				EB			WB			NB			SB											
85th % Speed / Rest in Walk / Corner Radius				0	No	25	0	No	25	0	No	25	0	No	25									
Walkway / Crosswalk Width / Length, ft				9.0	12	0	9.0	12	0	9.0	12	0	9.0	12	0									
Street Width / Island / Curb				0	0	No	0	0	No	0	0	No	0	0	No									
Width Outside / Bike Lane / Shoulder, ft				12	5.0	2.0	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0									
Pedestrian Signal / Occupied Parking				No	0.50	No	0.50	No	0.50	No	0.50	No	0.50											

HCS 2010 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB		Analysis Date	1/24/2017		Area Type	Other
Jurisdiction	DuPage County		Time Period	SAT Midday		PHF	0.95
Urban Street	63rd Street		Analysis Year	2018		Analysis Period	1 > 7:00
Intersection	63rd Street with Woodw...		File Name	63rd and Woodward SATFU.xus			
Project Description	Future SAT Midday Peak Hour						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	20	724	445	191	841	6	415	41	236	18	57	17

Signal Information				Signal Timing (s)									Signal Phases			
Cycle, s	110.0	Reference Phase	2													
Offset, s	0	Reference Point	Begin	Green	3.0	1.6	51.9	19.9	7.6	0.0						
Uncoordinated	No	Simult. Gap E/W	On	Yellow	3.0	3.0	4.5	4.5	4.5	0.0						
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.0	1.0	1.5	1.5	1.5	0.0						

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase	5	2	1	6		8		4
Case Number	1.1	3.0	1.1	4.0		9.0		12.0
Phase Duration, s	7.0	57.9	12.6	63.5		25.9		13.6
Change Period, (Y+R _c), s	4.0	6.0	4.0	6.0		6.0		6.0
Max Allow Headway (MAH), s	4.0	0.0	4.0	0.0		5.2		5.2
Queue Clearance Time (g _s), s	2.7		8.0			16.9		4.9
Green Extension Time (g _e), s	0.0	0.0	0.6	0.0		3.1		0.3
Phase Call Probability	1.00		1.00			1.00		0.95
Max Out Probability	0.00		0.00			0.40		0.00

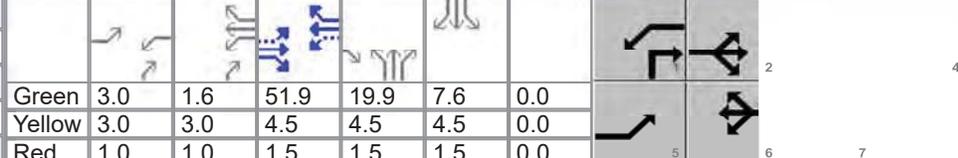
Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow Rate (v), veh/h	21	762	468	201	446	445	240	240	248	51		46
Adjusted Saturation Flow Rate (s), veh/h/ln	1723	1885	1594	1792	1863	1858	1810	1816	1610	1865		1775
Queue Service Time (g _s), s	0.7	11.3	15.9	6.0	11.7	11.8	13.8	13.7	14.9	2.9		2.7
Cycle Queue Clearance Time (g _c), s	0.7	11.3	15.9	6.0	11.7	11.8	13.8	13.7	14.9	2.9		2.7
Green Ratio (g/C)	0.50	0.47	0.65	0.57	0.52	0.52	0.18	0.18	0.26	0.07		0.07
Capacity (c), veh/h	352	1778	1041	467	973	971	328	329	418	129		122
Volume-to-Capacity Ratio (X)	0.060	0.429	0.450	0.431	0.459	0.459	0.732	0.728	0.595	0.396		0.375
Back of Queue (Q), ft/ln (95 th percentile)	12.2	189.6	225.9	104.8	191	189.3	270.7	276	251	68.4		58.8
Back of Queue (Q), veh/ln (95 th percentile)	0.5	7.5	9.0	4.2	7.5	7.6	10.8	10.8	10.0	2.6		2.4
Queue Storage Ratio (RQ) (95 th percentile)	0.10	0.00	0.65	0.31	0.00	0.00	0.89	0.00	1.39	0.00		0.00
Uniform Delay (d ₁), s/veh	14.3	12.9	9.4	12.7	9.7	9.8	42.5	42.5	35.7	49.0		48.9
Incremental Delay (d ₂), s/veh	0.1	0.8	1.4	0.6	1.6	1.6	5.2	5.1	1.9	2.8		2.7
Initial Queue Delay (d ₃), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
Control Delay (d), s/veh	14.4	13.6	10.8	13.3	11.3	11.4	47.8	47.5	37.6	51.8		51.6
Level of Service (LOS)	B	B	B	B	B	B	D	D	D	D		D
Approach Delay, s/veh / LOS	12.6		B	11.7		B	44.2		D	51.7		D
Intersection Delay, s/veh / LOS	20.8						C					

Multimodal Results	EB			WB			NB			SB		
Pedestrian LOS Score / LOS	2.8		C	2.3		B	2.9		C	3.0		C
Bicycle LOS Score / LOS	1.5		A	1.4		A	1.7		A	0.6		A

HCS 2010 Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	KLOA, Inc.			Duration, h	0.25		
Analyst	NJB	Analysis Date	1/24/2017		Area Type	Other	
Jurisdiction	DuPage County		Time Period	SAT Midday		PHF	0.95
Urban Street	63rd Street		Analysis Year	2018		Analysis Period	1 > 7:00
Intersection	63rd Street with Woodw...		File Name	63rd and Woodward SATFU.xus			
Project Description	Future SAT Midday Peak Hour						

Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	20	724	445	191	841	6	415	41	236	18	57	17

Signal Information																								
Cycle, s	110.0	Reference Phase	2	Green	3.0	1.6	51.9	19.9	7.6	0.0	Yellow	3.0	3.0	4.5	4.5	4.5	0.0	Red	1.0	1.0	1.5	1.5	1.5	0.0
Offset, s	0	Reference Point	Begin																					
Uncoordinated	No	Simult. Gap E/W	On																					
Force Mode	Fixed	Simult. Gap N/S	On																					

Saturation Flow / Delay	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Heavy Vehicle Adjustment Factor (f_{HV})	0.952	0.990	0.990	0.990	0.980	1.000	1.000	0.971	1.000	0.943	1.000	1.000
Approach Grade Adjustment Factor (f_g)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Parking Activity Adjustment Factor (f_p)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Lane Utilization Adjustment Factor (f_{LU})	1.000	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})	0.952	0.000		0.952	0.000		0.952	0.000		0.000	0.982	
Right-Turn Adjustment Factor (f_{RT})		0.000	0.847		0.997	0.997		0.000	0.847		0.931	0.934
Left-Turn Pedestrian Adjustment Factor (f_{LPB})	1.000			1.000			1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})			1.000			1.000			1.000			1.000
Movement Saturation Flow Rate (s), veh/h	1723	3770	1594	1792	3694	26	1810	1816	1610	694	2255	692
Proportion of Vehicles Arriving on Green (P)	0.03	0.63	0.47	0.08	0.70	0.52	0.18	0.18	0.18	0.07	0.07	0.07
Incremental Delay Factor (k)	0.11	0.50	0.50	0.11	0.50	0.50	0.18	0.18	0.15	0.15		0.15

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)	4.0	6.0	4.0	6.0		6.0		6.0
Green Ratio (g/C)	0.50	0.47	0.57	0.52		0.18		0.07
Permitted Saturation Flow Rate (s_p), veh/h/ln	604	0	708	0		1810		0
Shared Saturation Flow Rate (s_{sh}), veh/h/ln								
Permitted Effective Green Time (g_p), s	51.9	0.0	53.9	0.0		0.0		0.0
Permitted Service Time (g_u), s	43.6	0.0	40.6	0.0		0.0		0.0
Permitted Queue Service Time (g_{ps}), s	0.3		5.3					
Time to First Blockage (g_t), s	0.0	0.0	0.0	0.0		0.0		0.0
Queue Service Time Before Blockage (g_{ts}), s								
Protected Right Saturation Flow (s_R), veh/h/ln			1594			1610		
Protected Right Effective Green Time (g_R), s			19.9			8.6		

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	2.107	0.00	1.557	0.00	2.107	0.00	2.224	0.00				
Pedestrian F_s / F_{delay}	0.000	0.110	0.000	0.101	0.000	0.166	0.000	0.155				
Pedestrian M_{corner} / M_{cw}												
Bicycle c_b / d_b	943.42	15.35	1045.00	12.54	62.22	137.91	47.68					
Bicycle F_w / F_v	-3.64	1.03	-3.64	0.90	-3.64	1.20	-3.64	0.08				

--- Messages ---

WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

WARNING: The shared-plus-exclusive turn lane solution is an approximation of the HCM method, because more than three lane groups cannot be accommodated. Input data for Percent Turns in Shared Lane are used to specify proportion of turning vehicles in the shared lane.

--- Comments ---

Summary of Neighborhood Meeting
Meadowbrook Shopping Center, Downers Grove
Walgreens Development
November 28, 2017 7:00pm, Horizon Church, Meadowbrook Center

Attendance:

Owner: Perri Knight (project manager)

Residents/Tenants: No residents in attendance, 4 current tenants attended

Presentation materials included a site plan for Walgreens, a site improvement plan for overall center, renderings of Walgreens and façade, Walgreens provided statistics

Meeting began a few minutes after 7pm.

Questions related to timing of construction for façade improvements.

Answer: We are in for permit on the façade and hope to submit revised Walgreens plans for permit on 12/20. We hope to get approval from the village in February and start both WAG and facade in March.

Explained that due to the timing delay on getting the WAG approvals, WAG has renegotiated their lease/building and we are required to start the approval process over.

Questions related to roof replacements.

Answer: Unfortunately, do to the other aesthetic and ancillary improvements we are required to make, a full roof replacement is not in the budget.

Question re: Construction timing/staging

Answer: Perri committed to having another meeting with the tenants once we have approval on both projects to discuss staging for construction equipment as well as timelines for façade—e.g. starting at one end first and working our way down versus in the middle etc.

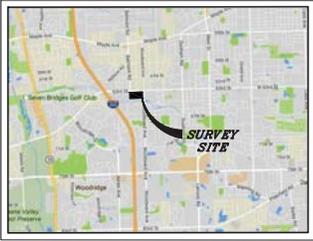
Question re: signage

Answer: Perri met with sign company who is working on proposal for entire center to unify signage with channel letters in accordance with village signage ordinances. Costs yet to be determined as is decision whether we move forward with new signage or keep the old.

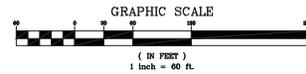
FINAL PLAT OF MEADOWBROOK ADDITION TO DOWNERS GROVE

BEING A SUBDIVISION OF PART OF THE NORTHEAST QUARTER OF SECTION 24, TOWNSHIP 38
NORTH, RANGE 10 EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPage COUNTY, ILLINOIS

PIN'S
08-24-202-005
08-24-202-006
08-24-202-009
08-24-203-004



LOCATION MAP
NOT TO SCALE



BASIS OF BEARINGS

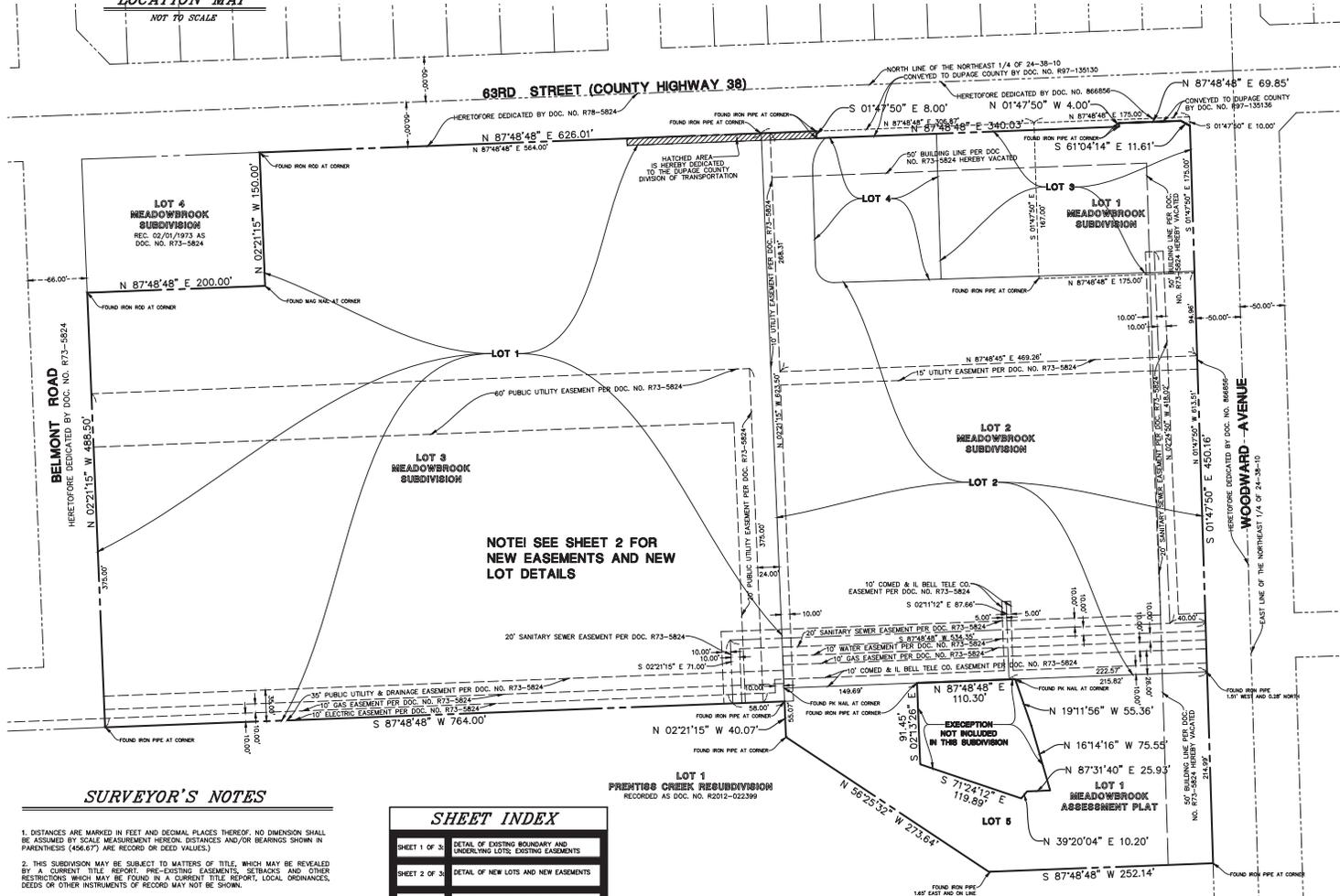
COORDINATES AND BEARINGS ARE BASED UPON THE ILLINOIS STATE PLANE COORDINATE SYSTEM, EAST ZONE (NAD 83), ADJUSTED TO GROUND VALUES, AS ESTABLISHED BY A REAL-TIME KINEMATIC (RTK) GLOBAL NAVIGATION SATELLITE SYSTEM (GNSS) UTILIZING THE TRIMBLE VRS NOW NETWORK.

SURVEY PREPARED FOR

FRONTLINE REAL ESTATE PARTNERS, LLC
707 SKOKIE BOULEVARD
NORTHBROOK, ILLINOIS 60062

SUBMITTED BY/RETURN TO:

THE VILLAGE OF DOWNERS GROVE
801 BURLINGTON AVENUE
DOWNERS GROVE, ILLINOIS 60515
PHONE: 630-434-5500



**NOTE! SEE SHEET 2 FOR
NEW EASEMENTS AND NEW
LOT DETAILS**

SURVEYOR'S NOTES

- DISTANCES ARE MARKED IN FEET AND DECIMAL PLACES THEREOF; NO DIMENSION SHALL BE ASSUMED BY SCALE MEASUREMENT HEREON. DISTANCES AND/OR BEARINGS SHOWN IN PARENTHESES (456.67') ARE RECORD OR DEED VALUES.
- THIS SUBDIVISION MAY BE SUBJECT TO MATTERS OF TITLE, WHICH MAY BE REVEALED BY A CURRENT TITLE REPORT. PRE-EXISTING EASEMENTS, SETBACKS AND OTHER RESTRICTIONS WHICH MAY BE FOUND IN A CURRENT TITLE REPORT, LOCAL ORDINANCES, DEEDS OR OTHER INSTRUMENTS OF RECORD MAY NOT BE SHOWN.
- THIS PROFESSIONAL SERVICE CONFORMS TO THE CURRENT ILLINOIS MINIMUM STANDARDS FOR A SUBDIVISION SURVEY. MANHARD CONSULTING, LTD. IS A PROFESSIONAL DESIGN FIRM, REGISTRATION NUMBER 184003350, EXPIRES APRIL 30, 2017.
- 5/8" DIA. IRON RODS WILL BE SET AT ALL PROPERTY CORNERS UNLESS OTHERWISE NOTED.

SHEET INDEX	
SHEET 1 OF 3	DETAIL OF EXISTING BOUNDARY AND UNDERLYING LOTS, EXISTING EASEMENTS
SHEET 2 OF 3	DETAIL OF NEW LOTS AND NEW EASEMENTS
SHEET 3 OF 3	CERTIFICATES, EASEMENT PROVISIONS

MANHARD CONSULTING, LTD. ALL RIGHTS RESERVED.

MEADOWBROOK FIRST RESUBDIVISION

DOWNERS GROVE, ILLINOIS

FINAL PLAT OF RESUBDIVISION

PROJ. NO.: TP

PROJ. ASSOC.: WM

DRAWN BY: WM-CM

DATE: 01/18/18

SCALE: 1"=60'

SHEET

1 OF 3

PREDC

FINAL PLAT ISSUED FOR REVIEW 01/18/18

